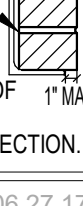






WALL FASTENING SCHEDULE			
CONNECTION: (NAIL SIZE AND POSITION EXAGGER FOR ILLUSTRATIVE PURPOSES)	FASTENER: MIN. NOMINAL LENGTH IN INCHES x MIN. NOMINAL NAIL DIA. IN INCHES	QUANTITY PER CONNECTION OR SPACING BETWEEN FASTENERS (INCHES ON CENTER)	
TOP OR SOLE PLATE (FACE NAIL)	3-1/2" x 0.162" NAIL (16d COMMON)	2	
DOUBLE 2x4 PLATE	3" x 0.131" NAILS	3	
	3" x 0.120" NAILS	4	
	3" x 0.148" NAIL (10d COMMON)	16" O.C.	
INTERIOR WALL SILL PLATE TO CONCRETE BEARING	3-1/2" x 0.162" NAIL (16d COMMON)		16" O.C.
	3" x 0.131" NAILS		12" O.C.
	3" x 0.120" NAILS		
INTERIOR WALL SILL PLATE TO WOOD BEARING	SEE WOOD BASE PLATE ATTACHMENT SCHEDULE ON S4.3		
	HILTI TX-CP72P823 POWDER ACTUATED FASTENERS W/23MM WASHERS	3'-0" O.C.	SEE WOOD BASE PLATE ATTACHMENT SCHEDULE ON SHEET S4.3
	3" x 0.131" NAIL		1'-0" O.C.
STUD TO TOP OR SOLE PLATE (TOE-NAIL)	3-1/2" x 0.162" NAIL (16d COMMON)	3	
	3" x 0.131" NAILS		
	3" x 0.120" NAILS	4	
CAPTOP PLATE LAPS AND INTERSECTIONS	3-1/2" x 0.162" NAIL (16d COMMON)	2 EACH SIDE OF LAP	
	3" x 0.131" NAILS	3 EACH SIDE OF LAP	
	3" x 0.120" NAILS		
DOUBLE STUD	3" x 0.148" NAIL (10d COMMON)	12" O.C.	
	3" x 0.131" NAILS		8" O.C.
	3" x 0.120" NAILS		
CORNER STUDS	3-1/2" x 0.162" NAIL (16d COMMON)	24" O.C.	
	3" x 0.131" NAILS	16" O.C.	
	3" x 0.120" NAILS	12" O.C.	
ROOF TOP PLATES TO HEADER	3" x 0.131" NAILS	(2) ROWS AT 6" O.C. UPPER PLATE TO LOWER PLATE (2) ROWS AT 6" O.C. LOWER PLATE TO HEADER	
FLOOR FASTENING SCHEDULE			
CONNECTION: (NAIL SIZE AND POSITION EXAGGER FOR ILLUSTRATIVE PURPOSES)	FASTENER: MIN. NOMINAL LENGTH IN INCHES x MIN. NOMINAL NAIL DIA. IN INCHES	QUANTITY PER CONNECTION OR SPACING BETWEEN FASTENERS (INCHES ON CENTER)	
JOIST TO SILL OR ORDER (TOE-NAILED)	FLOOR TRUSS TO TOP PLATE	3" x 0.148" NAIL (10d COMMON)	3
BLOCKING TO JOIST (LISTED NUMBER OF FASTENERS AT EACH END)	3" x 0.131" NAILS		4
	3" x 0.120" NAILS		2
	2-1/2" x 0.131" NAIL (Ø 131x33 COMMON)		3
BLOCKING BETWEEN JOIST OR RAFTER TO TOP PLATE (TOE-NAILED)	3" x 0.131" NAILS		3
	3" x 0.120" NAILS		4
	3-1/2" x 0.162" NAIL (16d COMMON)		3
STRONG BACK W/ 2x4 BLOCK	3" x 0.131" NAILS		4
	3" x 0.120" NAILS		
	3-1/2" x 0.162" NAIL (16d COMMON)		3
2x4 BLOCK NAILS AT TOP, BOTTOM, AND WEB CROSSING	3" x 0.131" NAILS		4
LEDGER STRIP	3-1/2" x 0.162" NAIL (16d COMMON)		3
	3" x 0.131" NAILS		4
	3" x 0.120" NAILS		
JOIST TO RIM JOIST	3-1/2" x 0.162" NAIL (16d COMMON)		3
	3" x 0.131" NAILS		5
	3" x 0.120" NAILS		6
RIM JOIST TO TOP PLATE (TOE-NAILED)	2-1/2" x 0.113" NAIL (Ø 131x33 BOX)		6" O.C.
	3-1/2" x 0.162" NAIL (16d COMMON)		8" O.C.
	3" x 0.131" NAILS		6" O.C.
ROOF FASTENING SCHEDULE			
CONNECTION: (NAIL SIZE AND POSITION EXAGGER FOR ILLUSTRATIVE PURPOSES)	FASTENER: MIN. NOMINAL LENGTH IN INCHES x MIN. NOMINAL NAIL DIA. IN INCHES	QUANTITY PER CONNECTION OR SPACING BETWEEN FASTENERS (INCHES ON CENTER)	
CEILING JOIST TO PLATE	3-1/2" x 0.162" NAIL (16d COMMON)	3	
ROOF RAFTER TO PLATE (TOE-NAILED)	3" x 0.131" NAILS		5
	3" x 0.120" NAILS		
	2-1/2" x 0.131" NAIL (Ø 131x33 COMMON)		3
ROOF RAFTER TO HIP (TOE-NAILED)	3-1/2" x 0.162" NAIL (16d COMMON)		3
	3" x 0.131" NAILS		4
	3" x 0.120" NAILS		
ROOF RAFTER TO RIDGE BEAM (FACE NAILED)	3-1/2" x 0.162" NAIL (16d COMMON)		2
	3" x 0.148" NAIL (10d COMMON)		3
	3" x 0.120" NAILS		4
ROOF RAFTER TO RIDGE BEAM (FACE NAILED)	3-1/2" x 0.162" NAIL (16d COMMON)		2
	3" x 0.131" NAILS		3
	3" x 0.120" NAILS		4
ROOF RAFTER TO RIDGE BEAM (FACE NAILED)	3-1/2" x 0.162" NAIL (16d COMMON)		2
	3" x 0.131" NAILS		3
	3" x 0.120" NAILS		4

FLOOR AND ROOF SHEATHING SCHEDULE				
SHEATHING	NAILING REQUIREMENTS			
23/32" APA RATED FLOOR SHEATHING	FLOORS			
	GLUED AND NAILED WITH Ø 131x33 COMMON SMOOTH AT 6" O.C. ALONG PANEL EDGES AND 12" O.C. ALONG INTERMEDIATE SUPPORTS. STAPLES ARE NOT ALLOWED. SEE NOTE 1.			
	[ALT.] GLUED AND NAILED WITH 6d RING OR SCREW SHANK NAILS AT 6" O.C. ALONG PANEL EDGES WHICH ALIGN WITH FLOOR TRUSSES AND/OR RIBBONS AND 12" O.C. ALONG INTERMEDIATE SUPPORTS. SEE NOTE 1.			
19/32" APA RATED OSB	PITCHED ROOFS			
	NAILED WITH Ø 131x33 COMMON SMOOTH OR DEFORMED SHANK NAILS AT 6" O.C. ALONG PANEL EDGES AND 12" O.C. ALONG INTERMEDIATE SUPPORTS. INSTALL CLIPS AT PANEL EDGES. STAPLES ARE NOT ALLOWED.			
	NOTES: 1. USE ONLY ADHESIVES CONFORMING TO APA SPECIFICATION AFG-01 OR ASTM D3498. APPLY IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. 2. THIS TABLE IS BASED UPON THE APA RECOMMENDATIONS FOR STURD-FLOOR AND SHEATHING ATTACHMENT			
MASONRY VENEER - LOOSE LINTEL SCHEDULE				
FOR EVERY 4" BRICK VENEER (40 PSF MAX.)				
OPENING SIZE	SIZE	MAX GAP	REMARK	
UP TO 4'-0"	L3 x 3-1/2 x 14"	1/2"	LLH	
4'-1" TO 5'-6"	L4 x 3-1/2 x 516"	1/2"	LLV	
5'-7" TO 7'-6"	L5 x 3-1/2 x 516"	1/2"	LLV	
7'-7" TO 9'-6"	L6 x 3-1/2 x 516"	1/2"	LLV	
NOTES: 1. LINTELS MUST BE INSTALLED WITH VERTICAL LEG 1/2" MAX. FROM BACK FACE OR BRICK. 2. L.L.H. = LONG LEG HORIZONTAL, L.L.V. = LONG LEG VERTICAL. 3. PROVIDE MIN. 6" BEARING EACH END TYPICAL. 4. PROVIDE BRICK/WALL ANCHORS AT 16" O.C. HORIZ. AT FIRST COURSE ABOVE LINTEL. 5. STEEL LINTELS, EXCEPT WHERE FABRICATED OF APPROVED CORROSION-RESISTANT STEEL OR OF STEEL HAVING A CORROSION RESISTANT OR OTHER APPROVED COATING, SHALL BE PROTECTED AGAINST CORROSION WITH AN APPROVED COAT OF PAINT, ENAMEL, OR OTHER APPROVED PROTECTION.				12" MAX GAP 
MULTIPLE MEMBER BEAM CONNECTIONS				
S				
MINIMUM EDGE, END, AND ROW SPACING REQUIREMENTS FOR SCREWS SHALL BE PER MANUFACTURER SPECIFICATIONS. 2. 1/2" Ø BOLTS SHALL HAVE 1/2" MIN. 916" MAX. Ø PRE-DRILLED HOLES WITH AN EDGE DISTANCE OF 2" MIN. FROM THE TOP OF BEAM AND 3" MIN. SPACING BETWEEN SCREWS.				
ALLOWABLE WOOD PRESERVATIVE SYSTEMS AND REQUIRED CORROSION PROTECTION				
LOCATION/USE	ALLOWABLE PRESERVATIVE - MIN. RETENTION (PCF)	MIN. CORROSION PROTECTION REQ'D (2)	FASTENERS	
SILL PLATES (ABOVE GROUND)	DOT AND POLYMER BINDER - 0.50 PCF (3) INORGANIC BORON (SBO) - SEE NOTE 1 ALKALINE COPPER QUAT. (ACQ-CAD) (NO AMMONIA) - 0.15 PCF	STANDARD G90 ZINC GALVANIZED COATING	STANDARD G90 ZINC GALVANIZED COATING	
	COPPER AZOLE (CA-B) - 0.10 PCF	STEEL LESS THAN 14 GA. ASTM A663 TYPE C185 ZINC GALVANIZED COATING OR STEEL 14 GAUGE AND THICKER	HOT-DIP GALVANIZED PER ASTM A153 OR EQUIVALENT	
	MICRONIZED COPPER QUAT. (MCQ) - 0.15 PCF			
EXTERIOR LUMBER ABOVE GROUND (JOISTS, DECKING, LEDGERS, ETC.)	ALKALINE COPPER QUAT. (ACQ-CAD) (NO AMMONIA) - 0.20 PCF			
	COPPER AZOLE (CA-B) - 0.10 PCF			
	MICRONIZED COPPER QUAT. (MCQ) - 0.15 PCF			
FIRE RETARDANT TREATED LUMBER (FRT) (4)	FRX AND THERMEX-FR (TM)			
	DRICON (TM)			
	PYRO-GUARD (TM)			
EXTERIOR LUMBER IN CONTACT WITH GROUND OR FRESH WATER OR EXPOSED TO SALT WATER	ALKALINE COPPER QUAT. (ACQ-CAD) (NO AMMONIA) - 0.40 PCF			
	COPPER AZOLE (CA-B) - 0.21 PCF			
	MICRONIZED COPPER QUAT. (MCQ) - 0.34 PCF	STAINLESS STEEL TYPES 303,304,305 OR 316		
NOTES: 1. S6X MAY BE USED ABOVE GROUND WHERE CONTINUOUSLY PROTECTED FROM EXPOSURE TO LIQUID WATER IN APPLICATIONS SUCH AS SILL PLATES OR OTHER ENCLOSED STRUCTURAL FRAMING AT RETENTIONS (B203 BASIS) OR 0.17 PCF OR 0.28 PCF WHERE FORMOSAN TERMITES ARE CONFIRMED. 2. CORROSION PROTECTION OF FASTENERS SHALL MATCH CORROSION PROTECTION OF STEEL ACCESSORY BEING ATTACHED. DO NOT USE STAINLESS STEEL WITH HDG CORROSION PROTECTION. 3. NOT TO BE USED IN AREAS OF FORMOSAN TERMITES. 4. DESIGN VALUE FOR FASTENERS USED WITH FRT LUMBER SHALL BE MULTIPLIED BY A 0.90 ADJUSTMENT FACTOR. 5. GENERAL CONTRACTOR SHALL PROVIDE LUMBER PRESERVATIVE, STEEL ACCESSORY AND FASTENER SUBMITTALS IN ACCORDANCE WITH THE SCHEDULE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.				
BUILDING SHRINKAGE AND SETTLEMENT SCHEDULE				
FRAMED FLOOR	SECOND	THIRD	ROOF	
TOTAL BUILDING MOVEMENT	3/8"	3/4"	1-1/8"	
NOTES: 1. VALUES ARE BASED ON ACCEPTED ENGINEERING PRACTICES AND TOLERANCES FOR WOOD FRAMING CONSTRUCTION. WEATHER AND OTHER ENVIRONMENTAL CONDITIONS MAY AFFECT ACTUAL SHRINKAGE VALUES. 2. VALUES CONSIDER SHRINKAGE OF WOOD FRAMING (STUDS/PLATES), TAKE UP OF FRAMING (CLOSING OF GAPS), AND CREEP (LONG TERM MOVEMENT UNDER SUSTAINED LOADING). 3. PLUMBER TO PROVIDE (1) CODE APPROVED EXPANSION JOINT AT EACH FLOOR ON ALL VERTICAL PVC RISERS WITH A "T" CONNECTION PENETRATING A 2x STUD TO COMPENSATE FOR BUILDING MOVEMENT. INSTALL PER MANUFACTURER'S SPECIFICATIONS.				

WOOD COLUMN SIZE LEGEND			
MARK	SIZE	MARK	SIZE
1-44	(1) 44	1-66	(1) 66
2-24	(2) 24	2-26	(2) 26
3-24	(3) 24	3-26	(3) 26
4-24	(4) 24	4-26	(4) 26
5-24	(5) 24	5-26	(5) 26
6-24	(6) 24	6-26	(6) 26
WOOD COLUMN SCHEDULE			
LEVELS SUPPORTED	MARK		
ROOF	C1	C2	C3
ROOF + 1 FLR ABOVE	(2) 24		
ROOF + 2 FLR ABOVE	(4) 24		
NOTES: 1. WHERE COLUMNS ARE SHOWN ON EITHER SIDE OF OPENING, JACK SCHEDULE REQUIREMENTS ARE NOT APPLICABLE FOR THAT OPENING. 2. ALL COLUMNS ARE ASSUMED TO BE SPF #2 GRADE OR BETTER, U.N.O. 3. NON-BEARING INTERIOR WALL STUDS MAY BE SPF-STUD GRADE 2x4 AT 24" O.C. 4. PRE-FAB STRUCTURAL COLUMNS - CONTRACTOR TO SUPPLY ENGINEER WITH SPECIFICATIONS AND DESIGN CRITERIA.			
JACK AND KING STUD SCHEDULE			
OPENING	ROOF BEARING ABOVE	ROOF AND 1 FLR BEARING ABOVE	ROOF AND 2 FLR BEARING ABOVE
EXTERIOR WALLS - TYPES E1 & E2			
0'-0" UP TO 5'-6"	1 J & 1 S	1 J & 1 S	1 J & 1 S
BREEZEWAY WALLS - TYPE B1			
0'-0" UP TO 3'-3"	1 J & 1 S	1 J & 1 S	1 J & 1 S
INTERIOR WALLS - TYPE I1			
0'-0" UP TO 3'-3"	—	1 J & 1 S	1 J & 1 S
INTERIOR WALLS - TYPE I2			
0'-0" UP TO 3'-3"	1 J & 1 S	1 J & 1 S	1 J & 1 S
NOTES: 1. TYPICAL UNLESS NOTED OTHERWISE ON PLANS. 2. J = JACK UNDER HEADER 3. S = FULL HEIGHT STUD NAILED TO JACK ALONGSIDE OF JACK 4. ALL JACKS AND STUDS TO BE 2x SPF #2 GRADE OR BETTER 5. ALL JACKS AND STUDS TO BE NAILED TOGETHER WITH Ø 131x33 NAILS AT 6" O.C. 6. FOR FLR BEAMS WITH NO COLUMNS LABELED ON THE PLAN, USE THE TOTAL NUMBER OF JACKS STUDS SHOWN IN THE SCHEDULE ABOVE, EXCEPT LOCATE THEM ALL UNDER THE BEAM, BLOCKED AS REQUIRED.			
BEARING WALL STUD SCHEDULE - SPF #1/#2			
LEVELS SUPPORTED	EXTERIOR - P	BREEZEWAY - P	INTERIOR - P
ROOF	E1 (1)	E2	B1 (1)
ROOF + 1 FLR ABOVE	26/16	26/16	26/16
ROOF + 2 FLR ABOVE	26/16	26/16	26/16
LEGEND: 26/16 = 2x4 AT 16" O.C. 24/12 = 2x4 AT 12" O.C. 24/12 = 2x4 AT 12" O.C., DOUBLE STUD UNDER TRUSS 26/12 = 2x6 AT 16" O.C. 24/12 = 2x4 AT 12" O.C. 26/12 = 2x6 AT 16" O.C. 24/12 = 2x4 AT 12" O.C. 26/12 = 2x6 AT 16" O.C.			
NOTES: 1. TYPICAL UNLESS NOTED OTHERWISE ON PLANS. 2. SCHEDULE IS BASED ON A NOMINAL 10'-8" FLOOR-TO-FLOOR HEIGHT, UNLESS NOTED OTHERWISE. 3. AT PLATFORM FRAMED WALLS: ALL STUDS SHALL ALIGN WITHIN 2" OF TRUSSES ABOVE WHERE STUDS ARE SPACED AT 12" O.C. AT FULL STORY FRAMED WALLS: IF A DOUBLE EVERY OTHER STUD IS SPECIFIED, DOUBLE STUD SHALL OCCUR CLOSEST TO THE TRUSS 4. DOUBLE AND TRIPLE STUDS SHALL BE NAILED TOGETHER WITH Ø 131x33 NAILS AT 6" O.C. 5. NON-BEARING INTERIOR WALL STUDS MAY BE SPF-STUD GRADE 2x4 AT 24" O.C. 6. INSTALL BLOCKING WITHIN THE MIDDLE THIRD OF THE STUD HEIGHT AT ALL BEARING WALL PRIOR TO WALL SUPPORTING ANY WEIGHT. COORDINATE LOCATION WITH DRYWALL INSTALLATION. 7. STUD SPACING AT 2x6 WET WALLS TO BE SINGLE STUD TO MATCH ADJACENT 2x4 WALL STUD SPACING. 8. TYPICAL FLOOR STUD HEIGHT IS EQUAL TO = ACTUAL FLOOR-TO-FLOOR HEIGHT - SHEATHING THICKNESS - # OF TOP AND BOTTOM PLATES x PLATE THICKNESS (MINUS FLOOR TRUSS DEPTH, IF PLATFORM FRAMED). 9. TYPICAL TOP LEVEL STUD HEIGHT (SUPPORTING ROOF ONLY) IS EQUAL TO = TRUSS BEARING HEIGHT - # OF FLOOR STUDS 10. FULL STORY FRAMED WALL, UNLESS NOTED OTHERWISE ON PLANS P. PLATFORM FRAMED WALL, UNLESS NOTED OTHERWISE ON PLANS			
EXTERIOR BALCONY JOIST AND LEDGER ATTACHMENT SCHEDULE CONDITION WITH OSB SHEATHING			
LOCATION (LL-DL)	SPAN LENGTH	JOIST SIZE AND SPACING (P.T. SP#2)	REQUIRED "SIMPSON" JOIST HANGER (4)
EXTERIOR BALCONY JOISTS (BOLL+100L)	UP TO 5'-0"	2x8 AT 24" O.C.	LUS26
NOTES: 1. 2x8 LEDGER FOR 2 ROWS 2. WOOD STRUCTURAL PANEL SHEATHING TO RUN CONTINUOUS BEHIND LEDGER 3. SIMPSON SDWS SCREWS ARE REQUIRED TO HAVE A 1-1/2" EDGE DISTANCE FROM THE TOP OF THE LEDGER AND 3/4" FROM THE BOTTOM, WITH FASTENERS SPACED EQUALLY IN BETWEEN. 4. 0.148x1-1/2" NAIL INTO LEDGER, 0.148x3" NAIL INTO JOIST, AT SKEW CONDITIONS. SIMPSON LSS28, SURL26 AND LSS28 MODELS ARE ACCEPTABLE. ALTERNATE LEDGER FASTENERS AND/OR JOIST HANGERS SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR FOR REVIEW AND APPROVAL. 5. ALL JOIST HANGERS AND LEDGER FASTENERS SHALL BE HOT-DIPPED GALVANIZED OR HAVE APPROVED PROTECTION FOR THE TYPE OF PRESSURE TREATED LUMBER BEING USED. 6. IF 5/4 "TREX" TYPE MATERIAL IS UTILIZED, JOISTS TO HAVE A 16" O.C. MAXIMUM SPACING.			
BREEZEWAY JOIST AND LEDGER ATTACHMENT SCHEDULE			
LOCATION (LL-DL)	SPAN LENGTH	JOIST SIZE AND SPACING (P.T. SP#2)	REQUIRED "SIMPSON" JOIST HANGER (4)
BREEZEWAY JOISTS (100L+100L)	UP TO 9'-0"	2x10 AT 16" O.C.	LUS28
NOTES: 1. LEDGER TO MATCH SIZE OF JOISTS, U.N.O. 2. GYPSUM SHEATHING TO RUN CONTINUOUS BEHIND LEDGER 3. SIMPSON SDWS SCREWS ARE REQUIRED TO HAVE A 1-1/2" EDGE DISTANCE FROM THE TOP OF THE LEDGER AND 3/4" FROM THE BOTTOM, WITH FASTENERS SPACED EQUALLY IN BETWEEN. 4. 0.148x1-1/2" NAIL INTO LEDGER, 0.148x3" NAIL INTO JOIST, ALTERNATE LEDGER FASTENERS AND/OR JOIST HANGERS SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR FOR REVIEW AND APPROVAL.			

CONCRETE STRENGTH AND DURABILITY REQUIREMENTS						
LOC.	STRUCTURAL MEMBERS	EXPOSURE CLASS	(1) MINIMUM 28 DAY COMPRESSIVE STRENGTH (PSI)	(3) MAXIMUM WATER/CEMENT RATIO (PERCENT)	(4) RECOMMENDED MINIMUM SLUMP (INCHES)	(4)
FOOTINGS						
BELOW GRADE	WITH BOTTOM OF FOOTING POURED AT REQUIRED FROST DEPTH	F2	4500	0.45	3	
	FOUNDATION WALLS					
	PROTECTED FULL HEIGHT,	C0 <sup>2</sup>	3000	N/A	3	
	PARTIALLY EXPOSED AT GRADE, PROTECTED FROM MOISTURE	F0 <sup>2</sup>	3000	N/A	3	
	PARTIALLY EXPOSED AT GRADE, NOT PROTECTED FROM MOISTURE	F1	3500	0.55	3	
BELOW GRADE	UNPROTECTED WALLS IN CONTACT WITH SOIL	F2	4500	0.45	3	
	UNPROTECTED WALLS IN CONTACT WITH MOISTURE OR SOIL AND DE-ICING CHEMICALS	F3/C2	5000	0.40	3	
	SLABS ON GRADE					
AT GRADE	INTERIOR HEATED, NOT IN DIRECT CONTACT WITH SOIL	C0 <sup>2</sup>	3000	N/A	4	
	INTERIOR CONCRETE EXPOSED TO OCCASIONAL MOISTURE AND DE-ICING CHEMICALS	C2	5000	0.40	4	
	INTERIOR UNHEATED, NOT IN DIRECT CONTACT WITH SOIL	F0 <sup>2</sup>	3000	N/A	4	
	EXTERIOR, NOT IN DIRECT CONTACT WITH SOIL, NOT EXPOSED TO DE-ICING CHEMICALS	F1	3500	0.55	4	
	EXTERIOR, IN DIRECT CONTACT WITH SOIL, EXPOSED TO DE-ICING CHEMICALS	F2	4500	0.45	4	
EXPOSED TO FREEZING	EXTERIOR, IN DIRECT CONTACT WITH SOIL, EXPOSED TO DE-ICING CHEMICALS	F3/C2	5000	0.40	4	
	1. EXPOSURE CLASS DESCRIPTIONS:					
	F0:	CONCRETE NOT EXPOSED TO FREEZING AND THAWING CYCLES AND PROTECTED FROM MOISTURE.				
	F1:	CONCRETE EXPOSED TO FREEZING AND THAWING CYCLES AND OCCASIONAL EXPOSURE TO MOISTURE.				
	F2:	CONCRETE EXPOSED TO FREEZING AND THAWING CYCLES AND IN CONTINUOUS CONTACT WITH MOISTURE.				
F3:	CONCRETE EXPOSED TO FREEZING AND THAWING CYCLES AND IN CONTINUOUS CONTACT WITH MOISTURE AND EXPOSED TO DEICING CHEMICALS.					
EXPOSED TO FREEZING	C0:	CONCRETE DRY AND PROTECTED FROM MOISTURE.				
	C1:	CONCRETE EXPOSED TO MOISTURE BUT NOT TO DEICING CHEMICALS.				
	C2:	CONCRETE EXPOSED TO MOISTURE AND DEICING CHEMICALS.				
	2. CONCRETE WHICH CAN BE EXPOSED TO CYCLES OF FREEZING-AND-THAWING DURING CONSTRUCTION SHALL BE AIR-ENTRAINED IN ACCORDANCE WITH THE TABLES BELOW WITH THE SAME AIR CONTENT AS FOR EXPOSURE CLASS F1.					
	3. THE CONCRETE STRENGTHS SPECIFIED ARE THE CODE REQUIRED MINIMUM, SEE THE PLANS AND DETAILS FOR HIGHER CONCRETE STRENGTH REQUIREMENTS.					
RECOMMENDED SLUMP APPLY WHEN VIBRATION IS USED TO CONSOLIDATE THE CONCRETE AND MAY BE INCREASED BY 1" FOR OTHER CONSOLIDATION METHODS. SLUMP MAY BE INCREASED WHEN CHEMICAL ADJUTURES ARE USED, PROVIDED THAT THE ADJUTURE TREATED CONCRETE HAS THE SAME OR LOWER WATER/CEMENT RATIO AND DOES NOT SHOW SEGREGATION POTENTIAL OR EXCESSIVE BLEEDING. CONCRETE SLUMP SHALL NOT BE LESS THAN 1 INCH, MEASURED SLUMP TOLERANCE SHALL BE +/- 1.5 MAXIMUM.						
TOTAL AIR CONTENT FOR CONCRETE EXPOSED TO CYCLES OF FREEZING-AND-THAWING						
NOMINAL MAXIMUM AGGREGATE SIZE, (INCHES)		AIR CONTENT, PERCENT				
		EXPOSURE CLASS, F1	EXPOSURE CLASSES, F2 AND F3			
3/8"		6.0%				7.5%
1/2"		5.5%				7.0%
3/4"		5.0%				6.5%
1"		4.5%				6.0%
AGGREGATE SHALL CONFORM TO ASTM C33 AND MAY RANGE FROM 3/8" TO 3/4" IN SIZE. CONCRETE IN EXPOSURE CLASSES F1, F2, OR F3 SHALL BE AIR ENTRAINED ACCORDING TO TABLE 16.3.3.1, A-0.518. THIS TABLE SHOWN IS FOR USUAL MAXIMUM AGGREGATE SIZES USED IN STRUCTURAL CONCRETE. MEASURED AIR CONTENT TOLERANCE SHALL BE +/- 1.5 MAXIMUM.						
HEADER AND BEAM SCHEDULE (1,4,6)						
MARK	SIZE	MARK	SIZE	MARK	SIZE	MARK
2-26 (2,5)	(2) 2x6	3-26 (2,5)	(3) 2x6			
2-28 (2,5)	(2) 2x8	3-28 (2,5)	(3) 2x8			
2-210 (2,5)	(2) 2x10	3-210 (2,5)	(3) 2x10			
2-212 (2,5)	(2) 2x12	3-212 (2,5)	(3) 2x12			
2-9LVL (2,7)	(2) 1 3/4" x 9'-10" LVL	3-9LVL (2,7)	(3) 1 3/4" x 9'-10" LVL			
2-12LVL (3,7)	(2) 1 3/4" x 11'-8" LVL	3-12LVL (3,7)	(3) 1 3/4" x 11'-8" LVL			
2-14LVL (3,7)	(2) 1 3/4" x 14' LVL	3-14LVL (3,7)	(3) 1 3/4" x 14' LVL			
2-16LVL (3,7)	(2) 1 3/4" x 16' LVL	3-16LVL (3,7)	(3) 1 3/4" x 16' LVL			
2-18LVL (3,7)	(2) 1 3/4" x 18' LVL	3-18LVL (3,7)	(3) 1 3/4" x 18' LVL			
NOTES:						
SEE STRUCTURAL NOTES FOR REQUIRED WOOD SPECIES AND GRADE.						
FOR TOP LOADED BEAM DEPTHS < 12" NAIL EACH PLY OF MULTIPLE MEMBERS WITH (2) ROWS O 131X3" NAILS AT 12" O.C. STAGGER NAILS ON OPPOSITE SIDE OF 3 PLY BEAMS U.N.O.						
FOR TOP LOADED BEAM DEPTHS > 12" NAIL EACH PLY OF MULTIPLE MEMBERS WITH (3) ROWS O 131X3" NAILS AT 12" O.C. STAGGER NAILS ON OPPOSITE SIDE OF 3 PLY BEAMS U.N.O.						
SIDE LOADED MULTIPLE LVL'S, BEAMS SHALL BE CONNECTED AS NOTED ON THE FRAMING PLANS OR PER THE MANUFACTURERS REQUIREMENTS FOR THE HIGHEST LOADING CONDITION LOCATED.						
PROVIDE 1/2" WOOD SHIM BETWEEN EACH PLY OF DIMENSIONAL LUMBER DOOR AND WINDOW HEADERS FRAMED WITH WALLS. OMIT SHIMS FOR STRUCTURAL COMPOSITE LUMBER (LVL, PSL) AND DIMENSIONAL LUMBER BEAMS EXPOSED TO WEATHER.						
ALL WOOD BEAMS EXPOSED TO WEATHER SHALL BE TREATED AND CONNECTED WITH FASTENERS PROPERLY TREATED FOR THE TYPE OF PRESSURE TREATMENT USED. SEE COVER SHEET, SECTION 5, NOTE F FOR ADDITIONAL REQUIREMENTS.						
ONE PCE PARALLEL PSL BEAMS OF EQUAL WIDTH MAY BE SUBSTITUTED FOR MULTIPLE LVL BEAMS. PRE-ENGINEERED COMPONENT HEADERS MAY BE SUBSTITUTED FOR THE HEADERS SHOWN. THE CONTRACTOR SHALL VERIFY ALL DRAWINGS SHOWN AND SEAL BY A PROFESSIONAL ENGINEER TO THE S.E.R. FOR APPROVAL PRIOR TO FABRICATION.						





DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION  
GRADE 60 UNCOATED REINFORCEMENT, NORMAL-WEIGHT CONCRETE

BAR SIZE	f'c = 3,000 psi		f'c = 4,000 psi		f'c = 5,000 psi		f'c = 6,000 psi	
	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb
#3	9	6	8	6	7	6	6	6
#4	11	8	10	7	9	6	8	6
#5	14	10	12	9	11	8	10	7
#6	17	12	15	10	13	9	12	9
#7	20	14	17	12	15	11	14	10
#8	22	16	19	14	17	12	16	11
#9	25	18	22	15	20	14	18	13
#10	28	20	25	17	22	16	20	14
#11	31	22	27	19	24	17	22	16

- NOTES:  
1. Ldb = DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION (INCHES).  
2. Ldb = Ldb UNLESS CONDITIONS OF NOTE 3 ARE SATISFIED.  
3. Ldb = 0.7 Ldb FOR #11 BARS AND SMALLER WHEN COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 1/2" INCHES AND FOR 90° HOOKS COVER ON BAR EXTENSION BEYOND HOOK IS NOT LESS THAN 2 INCHES.  
4. HOOKS ARE NOT CONSIDERED EFFECTIVE FOR DEVELOPING BARS IN COMPRESSION.

DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION  
GRADE 60 EPOXY COATED REINFORCEMENT, NORMAL-WEIGHT CONCRETE

BAR SIZE	f'c = 3,000 psi		f'c = 4,000 psi		f'c = 5,000 psi		f'c = 6,000 psi	
	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb
#3	10	7	9	6	8	6	7	6
#4	14	10	12	8	11	8	10	7
#5	17	12	15	10	13	9	12	9
#6	20	14	18	12	16	11	14	10
#7	24	17	20	14	18	13	17	12
#8	27	19	23	16	21	15	19	14
#9	30	21	26	18	23	17	21	15
#10	34	24	29	21	26	19	24	17
#11	38	26	33	23	29	21	27	19

- NOTES:  
1. Ldb = DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION (INCHES).  
2. Ldb = Ldb UNLESS CONDITIONS OF NOTE 3 ARE SATISFIED.  
3. Ldb = 0.7 Ldb FOR #11 BARS AND SMALLER WHEN COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 1/2" INCHES AND FOR 90° HOOKS COVER ON BAR EXTENSION BEYOND HOOK IS NOT LESS THAN 2 INCHES.  
4. HOOKS ARE NOT CONSIDERED EFFECTIVE FOR DEVELOPING BARS IN COMPRESSION.

DEVELOPMENT LENGTHS IN COMPRESSION (IN INCHES)

BAR SIZE	f'c = 3.0 ksi		f'c = 3.5 ksi		f'c = 4.0 ksi		f'c = 4.5 ksi		f'c = 5.0 ksi		f'c = 6.0 ksi	
	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb	Ldb	0.7Ldb
#4	9	8	8	8	8	8	8	8	8	8	8	8
#5	11	11	10	10	9	9	9	9	9	9	9	9
#6	14	13	12	12	12	12	12	12	12	12	12	12
#7	17	16	15	14	14	14	14	14	14	14	14	14
#8	20	18	17	16	16	16	16	16	16	16	16	16
#9	22	21	19	18	18	18	18	18	18	18	18	18
#10	25	23	22	21	21	21	21	21	21	21	21	21
#11	28	26	25	23	23	23	23	23	23	23	23	23
#14	31	29	27	26	26	26	26	26	26	26	26	26

TENSION DEVELOPMENT AND LAP SPlice LENGTHS FOR UNCOATED BARS

BAR SIZE	LAP CLASS	f'c = 3,000 psi				f'c = 4,000 psi				f'c = 4,500 psi				f'c = 5,000 psi				f'c = 6,000 psi			
		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	A	22	33	17	25	19	28	15	22	18	27	14	21	17	25	13	20	16	23	12	18
	B	28	42	22	33	25	37	19	28	23	35	18	27	22	33	17	25	20	30	16	23
#4	A	29	43	22	33	25	37	19	29	24	35	18	27	23	34	17	26	21	31	16	24
	B	38	56	29	43	33	49	25	37	31	46	24	35	29	44	23	34	27	40	21	31
#5	A	36	54	28	42	31	47	24	36	30	44	23	34	28	42	22	32	26	38	20	30
	B	47	70	36	54	41	61	31	47	38	57	30	44	36	54	28	42	33	50	26	38
#6	A	43	65	33	50	43	56	29	43	35	53	27	41	34	50	26	39	31	46	24	35
	B	56	84	43	65	49	73	37	56	46	69	35	53	44	65	34	50	40	59	31	46
#7	A	63	94	48	72	54	81	42	63	51	77	40	59	49	73	38	56	45	67	34	51
	B	81	122	63	94	71	106	54	81	67	100	51	77	63	95	49	73	58	86	45	67
#8	A	72	107	55	83	62	93	48	72	59	88	45	68	56	83	43	64	51	76	39	59
	B	93	139	72	107	81	121	62	93	76	114	59	88	72	108	56	83	66	99	51	76
#9	A	81	121	62	93	70	105	54	81	66	99	51	76	63	94	48	72	57	86	44	66
	B	105	157	81	121	91	136	70	105	89	128	66	99	81	122	63	94	74	111	57	86
#10	A	91	136	70	105	79	118	61	91	74	111	57	86	71	106	54	81	64	96	50	74
	B	118	177	91	136	102	153	79	118	96	144	74	111	92	137	71	106	84	125	64	96
#11	A	101	151	78	116	87	131	67	101	82	123	64	95	78	117	60	90	71	107	55	82
	B	131	196	101	151	114	170	87	131	107	160	82	123	102	152	78	117	93	139	71	107

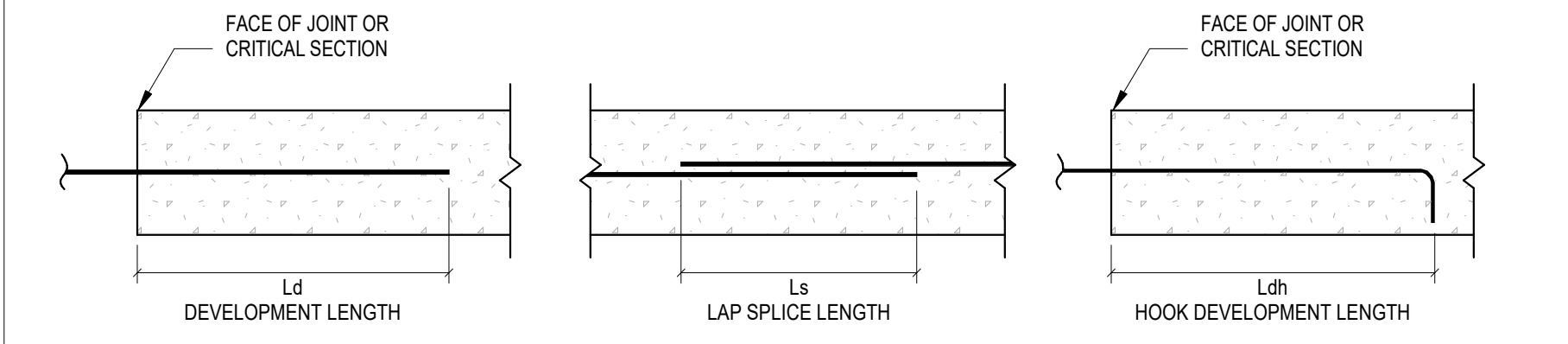
- NOTES:  
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.  
2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPlice LENGTHS ARE BASED ON ACI-318.  
3. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:  
BEAMS OR COLUMNS CASE 1 COVER AT LEAST 1db AND c <= SPACING AT LEAST 2 db  
CASE 2 COVER LESS THAN 1db AND c <= SPACING AT LEAST 2 db  
ALL OTHERS CASE 1 COVER AT LEAST 1db AND c <= SPACING AT LEAST 3 db  
CASE 2 COVER LESS THAN 1db AND c <= SPACING AT LEAST 3 db  
4. LAP CLASS A VALUES ARE THE REQUIRED TENSION DEVELOPMENT LENGTHS, Ld. LAP SPlice LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS;  
CLASS A - 1.3Ld AND CLASS B - 1.3Ld.  
5. "TOP BARS" ARE DEFINED PER ACI HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPlice. "OTHER BARS" ARE ALL BARS FOR WHICH THIS DOES NOT APPLY.  
6. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3.  
7. WHEN TENSION LAPPING SMALLER DIAMETER BAR WITH LARGER DIAMETER BAR, USE THE LAP LENGTH OF THE LARGER BARS.

TENSION DEVELOPMENT AND LAP SPlice LENGTHS FOR EPOXY COATED BARS

BAR SIZE	LAP CLASS	f'c = 3,000 psi				f'c = 4,000 psi				f'c = 4,500 psi				f'c = 5,000 psi				f'c = 6,000 psi			
		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	A	28	42	25	37	25	37	22	33	23	35	21	31	22	33	20	29	20	30	18	27
	B	37	55	33	49	32	48	28	42	30	45	27	40	29	43	25	38	26	39	23	34
#4	A	38	56	33	50	33	49	29	43	31	46	27	41	29	44	26	39	27	40	24	35
	B	49	73	43	65	42	63	37	56	40	60	35	53	38	57	34	50	35	52	31	46
#5	A	47	70	42	62	41	61	36	54	39	58	34	51	37	55	32	48	33	50	30	44
	B	61	91	54	81	53	79	47	70	50	75	44	66	47	71	42	63	43	65	38	57
#6	A	56	84	50	74	49	73	43	65	46	69	41	61	44	65	39	58	40	60	35	53
	B	73	109	65	97	63	95	56	84	60	89	53	79	57	85	50	75	52	78	46	68
#7	A	82	123	72	108	71	106	63	94	67	100	59	89	64	95	56	84	58	87	51	77
	B	106	159	94	141	92	138	81	122	87	130	77	115	83	124	73	109	75	113	67	100
#8	A	94	140	83	124	81	121	72	107	77	115	68	101	73	109	64	96	66	99	59	88
	B	122	182	107	161	105	158	93	139	99	149	88	131	94	141	83	125	86	129	76	114
#9	A	106	158	93	140	91	137	81	121	86	129	76	114	82	123	72	108	75	112	66	99
	B	137	205	121	181	119	178	105	157	112	169	99	148	106	159	94	140	97	145	86	128
#10	A	119	178	105	157	103	154	91	136	97	145	86	128	92	138	81	122	84	126	74	111
	B	154	231	136	204	134	200	118	177	126	189	111	167	120	179	106	158	109	164	96	144
#11	A	132	197	116	174	114	171	101	151	108	161	95	142	102	153	90	135	93	140	82	123
	B	171	257	151	226	148	222	131	196	140	210	123	185	133	199	117	175	121	182	107	160

- NOTES:  
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.  
2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPlice LENGTHS ARE BASED ON ACI-318.  
3. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:  
BEAMS OR COLUMNS CASE 1 COVER AT LEAST 1db AND c <= SPACING AT LEAST 2 db  
CASE 2 COVER LESS THAN 1db AND c <= SPACING AT LEAST 2 db  
ALL OTHERS CASE 1 COVER AT LEAST 1db AND c <= SPACING AT LEAST 3 db  
CASE 2 COVER LESS THAN 1db AND c <= SPACING AT LEAST 3 db  
4. LAP CLASS A VALUES ARE THE REQUIRED TENSION DEVELOPMENT LENGTHS, Ld. LAP SPlice LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS;  
CLASS A - 1.3Ld AND CLASS B - 1.3Ld.  
5. "TOP BARS" ARE DEFINED PER ACI HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPlice. "OTHER BARS" ARE ALL BARS FOR WHICH THIS DOES NOT APPLY.  
6. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3.  
7. WHEN TENSION LAPPING SMALLER DIAMETER BAR WITH LARGER DIAMETER BAR, USE THE LAP LENGTH OF THE LARGER BARS.

LAP SPlice DEVELOPMENT LENGTHS LEGEND

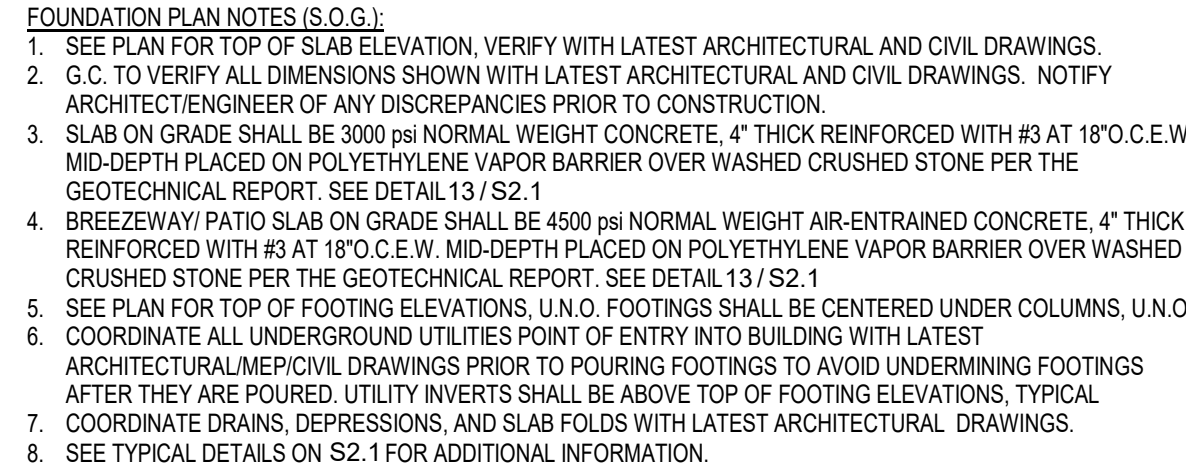


Seal



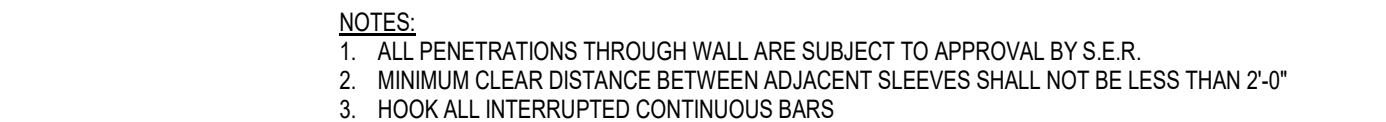




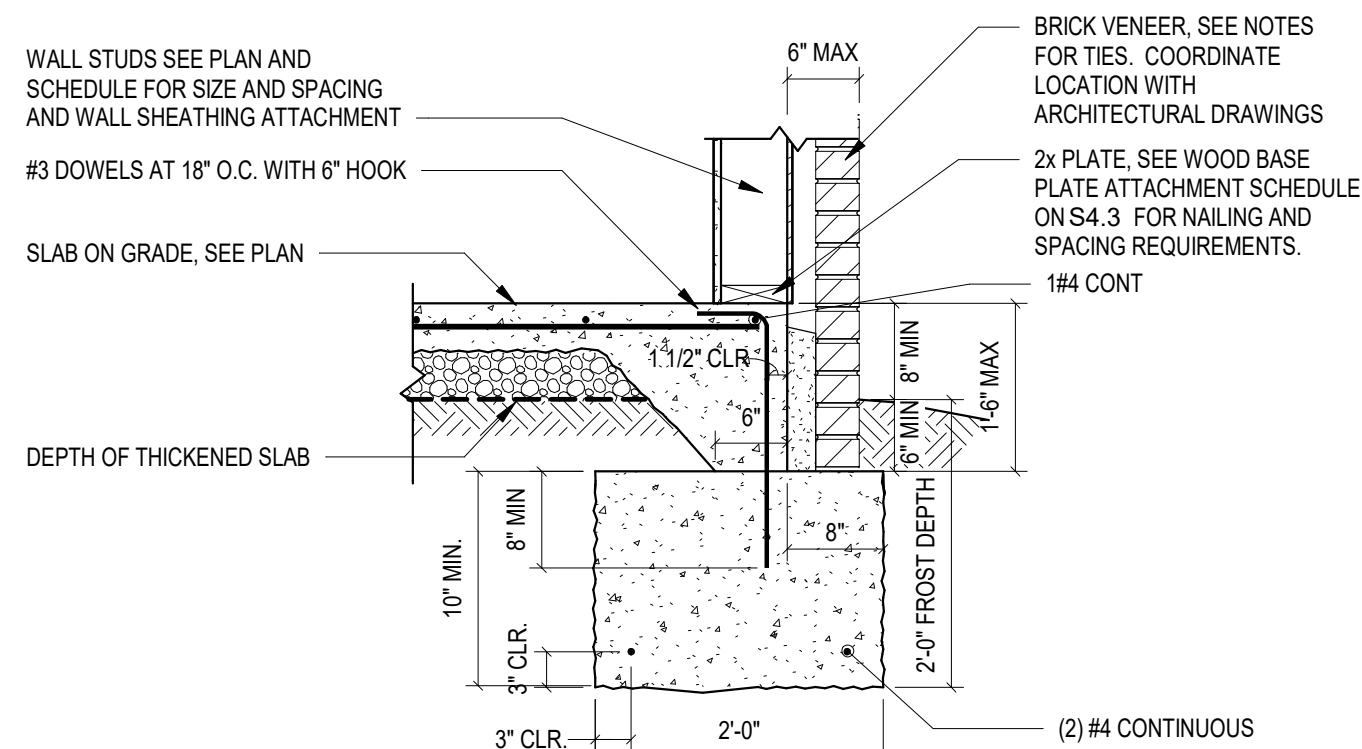


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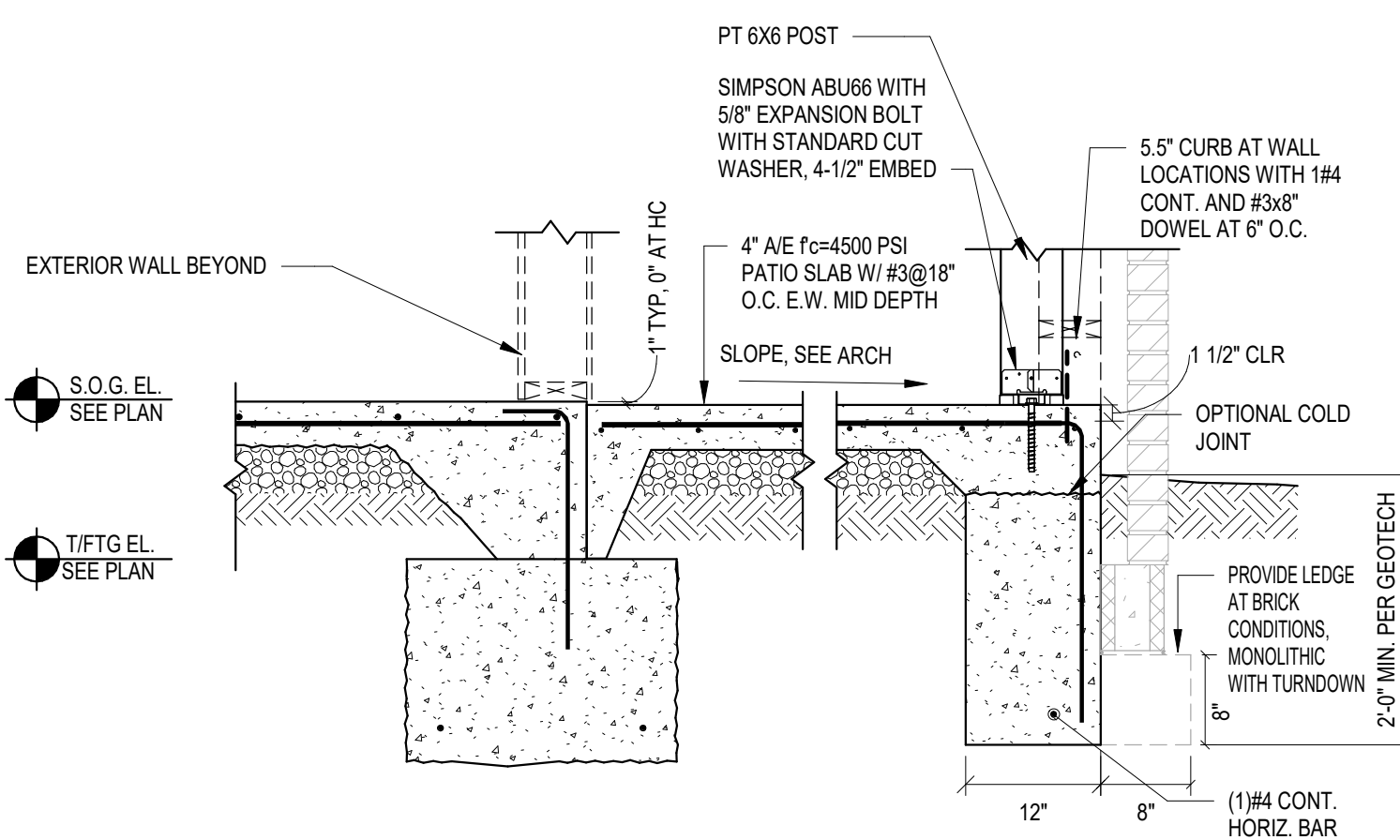




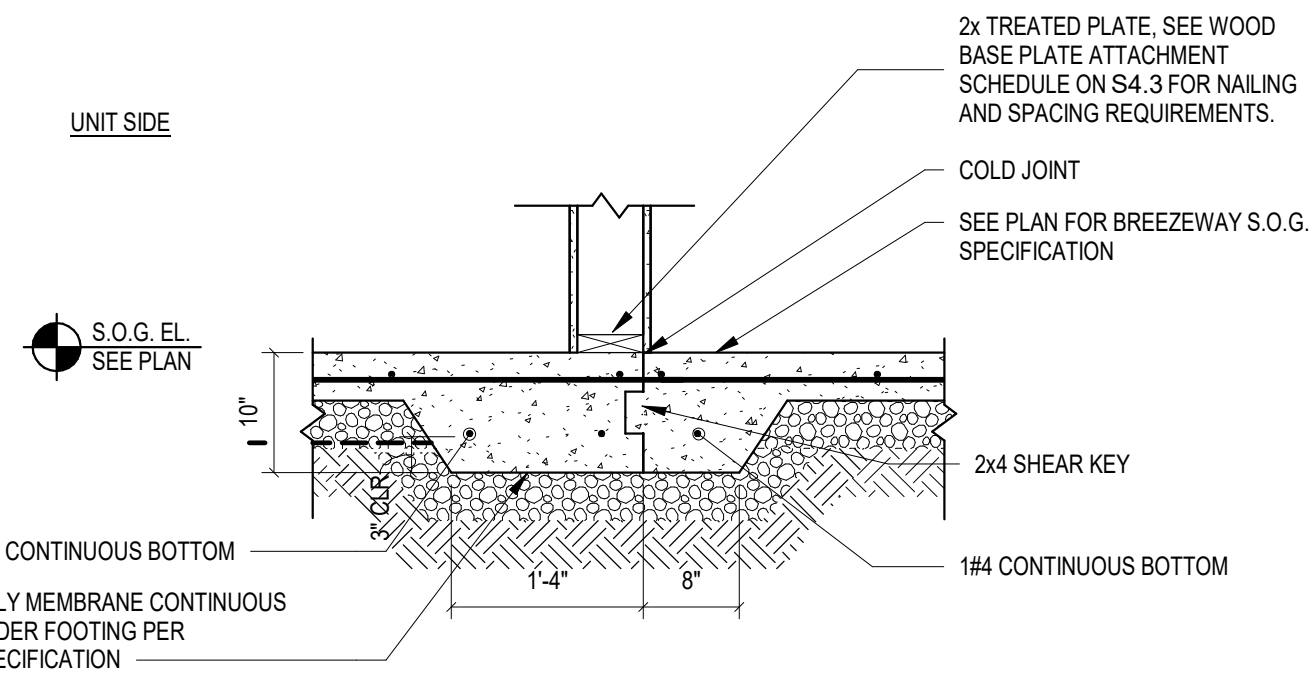
**NOTES:**

1. REQUIRED FROST DEPTH AND SOIL BEARING CAPACITY SHALL BE VERIFIED BY ON SITE GEOTECHNICAL ENGINEER PRIOR TO POURING FOOTING
2. FOOTING SHALL BE POURED DIRECTLY AGAINST UNDISTURBED EARTH

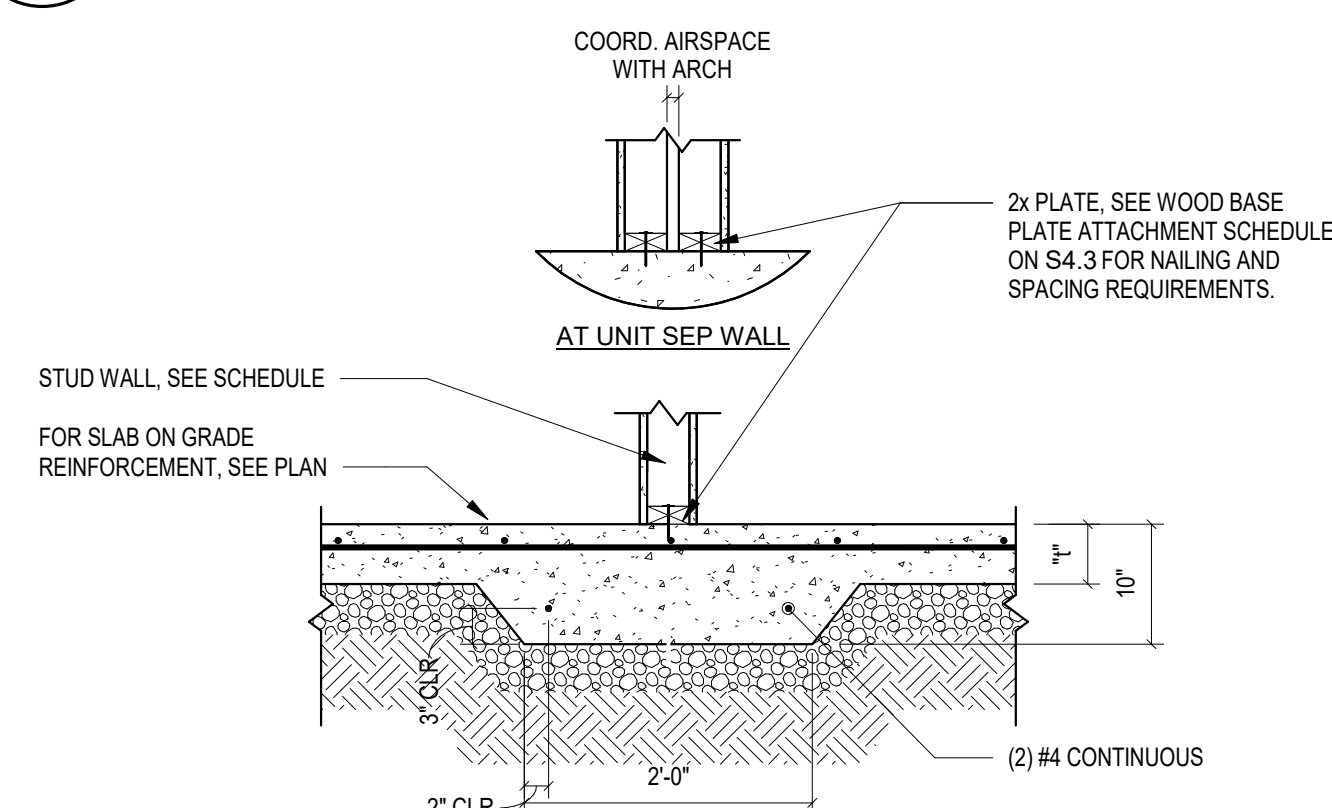
1 TYPICAL FOOTING AT EXTERIOR WALL



2 EXTERIOR FOUNDATION WALL AT PATIO



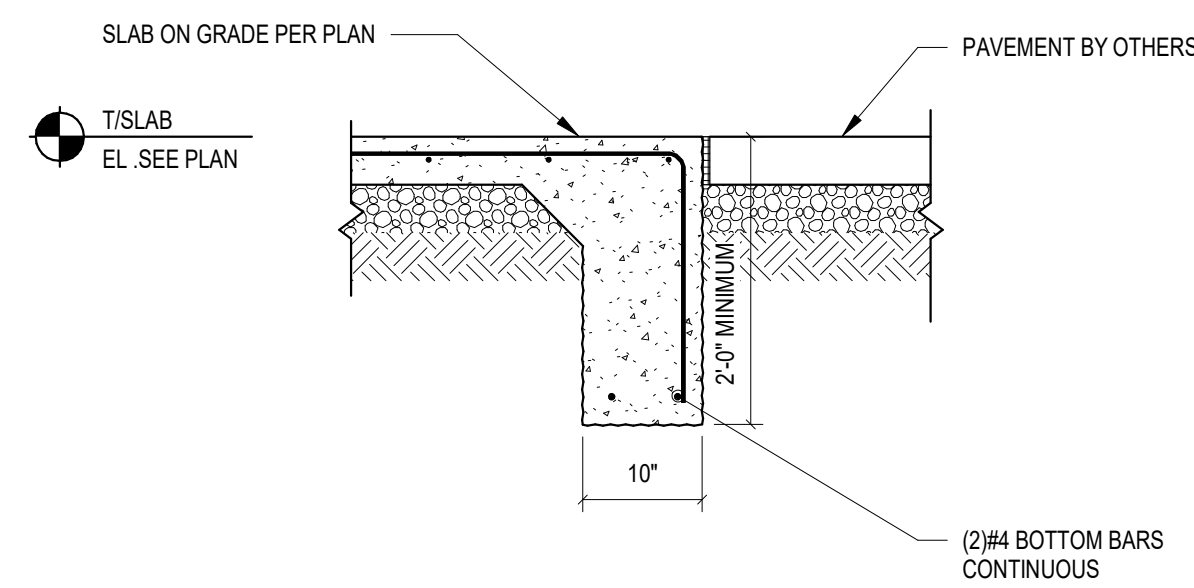
### 3 TYPICAL FOOTING AT BREEZEWAY



NOTES:

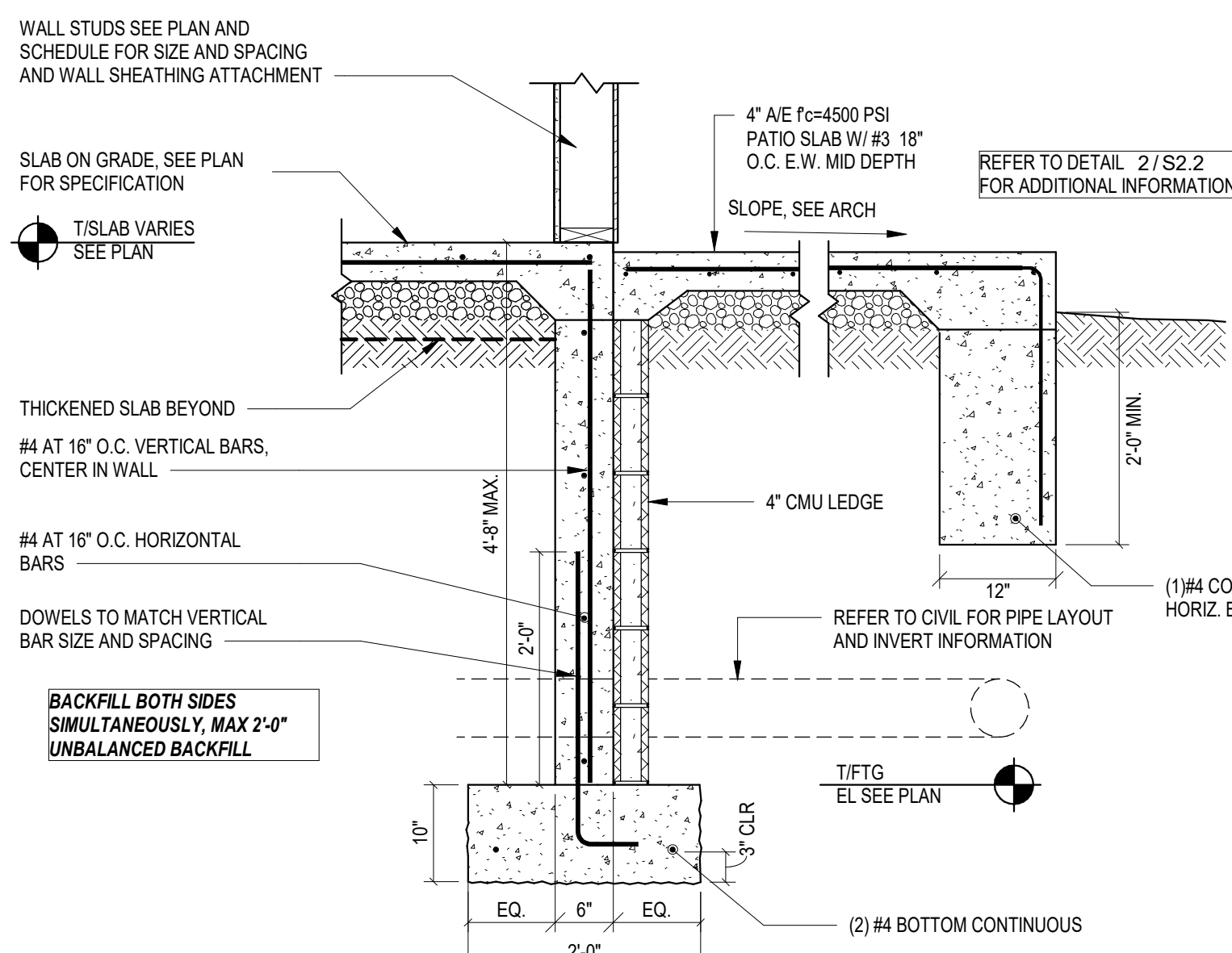
1. "T" = SLAB THICKNESS, SEE PLAN
2. THICKENED SLAB SUBGRADE SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER FOR ALLOWABLE BEARING CAPACITY AND SUBGRADE MODULUS JUST PRIOR TO PLACING CONCRETE. PROVIDE REPORT TO S.E.R. FOR REVIEW AND APPROVAL.
3. SLAB THICKNESS IS BASED ON THE SOIL MODULUS AND THE DESIGN METHODS PROPOSED BY THE U.S.ARMY, DOCUMENT TM-5-809-12.

#### 4 THICKENED SLAB AT INT. BEARING WALL



5 TURN DOWN SLAB ON GRADE

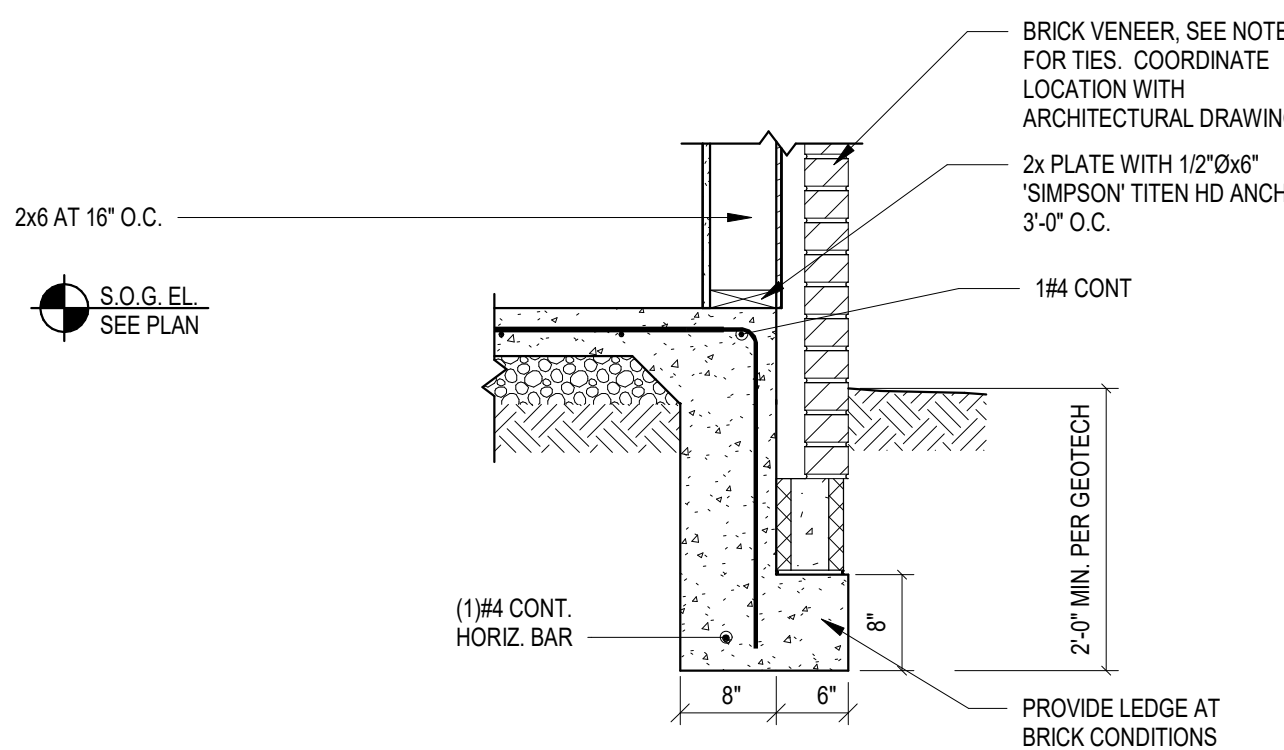
## 7 BURIED FOOTING AT UTILITY INVERTS



**NOTES:**

1. REQUIRED FROST DEPTH AND SOIL BEARING CAPACITY SHALL BE VERIFIED BY ON SITE GEOTECHNICAL ENGINEER PRIOR TO POURING FOOTING
2. FOOTING SHALL BE POURED DIRECTLY AGAINST UNDISTURBED EARTH

## 8 BURIED FOOTING AT UTILITY INVERTS AT PATIO



## 9 EXTERIOR FOUNDATION WALL AT PUMP ROOM



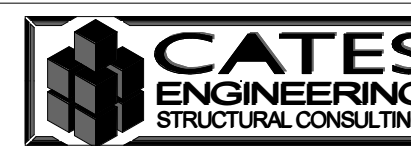
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CADD File:  
Drawn By: RCR/MRP  
Checked By: MRP

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Permit Release:  
February 19, 2026  
Construction Release Set

Revisions		
No.	Date	Description

ASI / RFI Revisions		
No.	Date	Description



13675 Heathcote Blvd. Suite 170 Gainesville, VA 20155  
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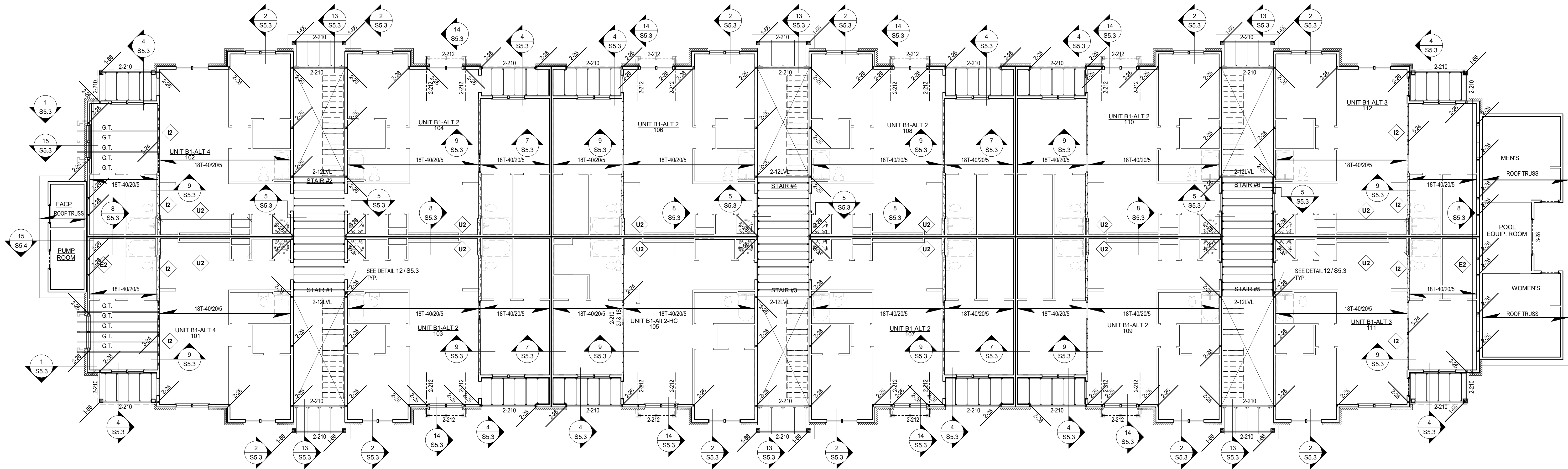
CATES ENGINEERING, Ltd. RESERVES ITS LAWFUL RIGHTS TO THESE DRAWINGS. THESE DRAWINGS ARE PREPARED FOR THIS PROJECT ONLY AND MAY NOT BE USED FOR ANOTHER PROJECT WITHOUT THE WRITTEN CONSENT OF CATES ENGINEERING, Ltd. DO NOT SCALE DRAWINGS. USE DIMENSIONS

**Daleville Town Center Apartments III**  
an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:  
**FOUNDATION DETAILS**

## S2.2





SECOND FLOOR FRAMING PLAN - BUILDING 1

SCALE: 1/8" = 1'-0"

- FLOOR FRAMING NOTES (WOOD):
1. ALL FLOOR FRAMING SHALL BE 18" DEEP PRE-ENGINEERED OPEN WEB WOOD TRUSSES DESIGNED AND SPACED BY TRUSS MANUFACTURER AT 24" O.C. MAX. U.N.O. FOR THE DESIGN LOADS.
  2. FLOOR TRUSS MANUFACTURER TO PROVIDE BLOCKOUTS FOR MECHANICAL DUCTS. VERIFY SIZE AND LOCATION OF FLOOR OPENINGS WITH LATEST ARCHITECTURAL AND MEP DRAWINGS.
  3. ALL TRUSS TO TRUSS CONNECTIONS TO BE SPECIFIED BY TRUSS MANUFACTURER.
  4. EXTERIOR DECK MEMBERS SHALL BE P.T. SPW2 MATERIAL. REFER TO BALCONY FRAMING SCHEDULE FOR JOIST AND LEDGER SIZE AND ATTACHMENT.
  5. PROVIDE (1) 2x6 STRONGBACK AT APPROXIMATE MID-SPAN OF THE TRUSSES UNDER 20'-0" OR APPROXIMATE THIRD POINTS OF TRUSSES OVER 20'-0". MAINTAIN 10'-0" MAX. SPACING.
  6. FOR STACKING NON BEARING WALLS FRAMED PARALLEL WITH FLOOR TRUSSES, PROVIDE 2x4 CRIPPLE STUDS AT 24" O.C. IN FLOOR PLENUM BETWEEN WALLS. SEE DETAIL 9/ S5.1.
  7. SEE BEARING WALL SCHEDULE ON SHEET S0.2 FOR STUD QUANTITY AND SPACING.
  8. EXTERIOR WALLS TO BE TYPE "E1" U.N.O. INTERIOR UNIT BEARING WALLS TO BE TYPE "I1" U.N.O. BREEZEWAY WALLS TO BE TYPE "C1" U.N.O. UNIT SEPARATION WALLS TO BE TYPE "U1" U.N.O. AND WET WALLS TO BE TYPE "W1" U.N.O.
  9. PRE-ENGINEERED STAIRS BY OTHERS. FABRICATOR TO SUBMIT SHOP DRAWINGS FOR APPROVAL BY ARCHITECT AND ENGINEER.
  10. REFER TO S5.1 FOR TYPICAL FRAMING DETAILS.

TYPICAL FLOOR HEADERS (U.N.O.)			
	LOCATION	MARK	MAXIMUM OPENING
1	EXTERIOR		
2	EXTERIOR AT BALCONY	TRUSS HEADER, SEE FRAMING DETAILS	
3	INTERIOR	2x8	3'-1"
4	BREEZEWAY	TRUSS HEADER, SEE FRAMING DETAILS	

NOTES:  
1. FOR DOUBLE AND TRIPLE WINDOWS, ALL HEADERS ARE CONTINUOUS MULTI-SPAN HEADERS WITH (2) 2x6 POSTS BETWEEN U.N.O.  
2. MAXIMUM OPENING CORRELATES TO ROUGH OPENING DIMENSION.

FLOOR FRAMING LEGEND			
	FLUSH TRUSS HEADER		GIRDER TRUSS
	FLUSH 2x HEADER		TRUSS SPAN DIRECTION
	DROPPED 2x HEADER		WALL TYPE
	INTERIOR UNIT BEARING WALL		ADDITIONAL LOADING SEE SHEET S0.1

WOOD TRUSS LEGEND (U.N.O.)			
	TYPE	LOADING	HATCH
	RESIDENTIAL	18T-40Z05	NONE
	PUBLIC AREA	18T-100Z05	
	LIGHT STORAGE/ MECH.	18T-125Z05	

NOTES:  
1. AREAS ON PLAN INDICATE DIFFERENCES IN TRUSS DEPTH OR LOADING. SEE PLAN FOR TRUSS DESIGN CRITERIA.

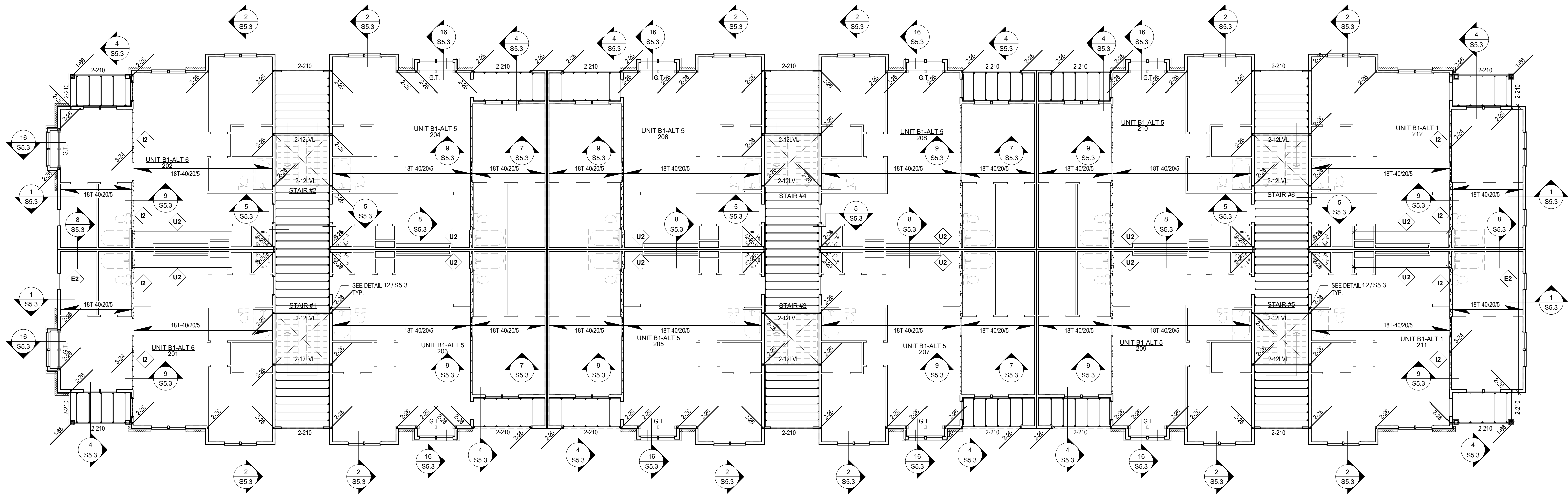
**DISCLAIMER NOTES**

A. THE CONTRACTOR, BEFORE STARTING ANY WORK, SHALL CHECK ALL DIMENSIONS GIVEN ON THE STRUCTURAL DRAWINGS, RELATING TO GRID LINES, COLUMN AND WALL LOCATIONS, STRUCTURAL AND FINISHED FLOOR ELEVATIONS, MEMBER SIZES, ETC. WITH THE ARCHITECTURAL AND CIVIL DRAWINGS. IF ANY DISCREPANCIES ARE NOTICED, IMMEDIATELY NOTIFY THE SER/ARCHITECT, AND WORK SHALL NOT COMMENCE UNTIL INSTRUCTIONS ARE RECEIVED FROM THE SER/ARCHITECT.

B. THE CONTRACTOR SHALL REFER TO THE SER/ARCHITECT FOR ANY DIMENSION NOT GIVEN ON OR NOT OBTAINABLE FROM THE CONSTRUCTION DRAWINGS. THE CONTRACTOR SHALL NOT USE SCALE TO OBTAIN OR VERIFY ANY DIMENSION SHOWN ON THESE DRAWINGS.



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THIRD FLOOR FRAMING PLAN - BUILDING 1

SCALE: 1/8" = 1'-0"

FLOOR FRAMING NOTES (WOOD):

- ALL FLOOR FRAMING SHALL BE 18" DEEP PRE-ENGINEERED OPEN WEB WOOD TRUSSES DESIGNED AND SPACED BY TRUSS MANUFACTURER AT 24" O.C. MAX. U.N.O. FOR THE DESIGN LOADS.
- FLOOR TRUSS MANUFACTURER TO PROVIDE BLOCKOUTS FOR MECHANICAL DUCTS. VERIFY SIZE AND LOCATION OF FLOOR OPENINGS WITH LATEST ARCHITECTURAL AND MEP DRAWINGS.
- ALL TRUSS TO TRUSS CONNECTIONS TO BE SPECIFIED BY TRUSS MANUFACTURER.
- EXTERIOR DECK MEMBERS SHALL BE P.T. SPH2 MATERIAL. REFER TO BALCONY FRAMING SCHEDULE FOR JOIST AND LEDGER SIZE AND ATTACHMENT.
- PROVIDE (1) 2x6 STRONGBACK AT APPROXIMATE MID-SPAN OF THE TRUSSES UNDER 20'-0" OR APPROXIMATE THIRD POINTS OF TRUSSES OVER 20'-0". MAINTAIN 10'-0" MAX. SPACING.
- FOR STACKING NON BEARING WALLS FRAMED PARALLEL WITH FLOOR TRUSSES, PROVIDE 2x4 CRIPPLE STUDS AT 24" O.C. IN FLOOR PLENUM BETWEEN WALLS. SEE DETAIL 5/ S5.1.
- SEE BEARING WALL SCHEDULE ON SHEET S0.2 FOR STUD QUANTITY AND SPACING.
- EXTERIOR WALLS TO BE TYPE "E1" U.N.O. INTERIOR UNIT BEARING WALLS TO BE TYPE "I1" U.N.O. BREEZEWAY WALLS TO BE TYPE "C1" U.N.O. UNIT SEPARATION WALLS TO BE TYPE "U1" U.N.O. AND WET WALLS TO BE TYPE "W1" U.N.O.
- PRE-ENGINEERED STAIRS BY OTHERS. FABRICATOR TO SUBMIT SHOP DRAWINGS FOR APPROVAL BY ARCHITECT AND ENGINEER.
- REFER TO S5.1 FOR TYPICAL FRAMING DETAILS.

TYPICAL FLOOR HEADERS (U.N.O.)			
	LOCATION	MARK	MAXIMUM OPENING
1	EXTERIOR		
2	EXTERIOR AT BALCONY	TRUSS HEADER, SEE FRAMING DETAILS	
3	INTERIOR	2x8	3'-1"
4	BREEZEWAY	TRUSS HEADER, SEE FRAMING DETAILS	

NOTES:  
1. FOR DOUBLE AND TRIPLE WINDOWS, ALL HEADERS ARE CONTINUOUS MULTI-SPAN HEADERS WITH (2) 2x6 POSTS BETWEEN U.N.O.  
2. MAXIMUM OPENING CORRELATES TO ROUGH OPENING DIMENSION.

FLOOR FRAMING LEGEND			
	FLUSH TRUSS HEADER		GIRDER TRUSS
	FLUSH 2x HEADER		TRUSS SPAN DIRECTION
	DROPPED 2x HEADER		WALL TYPE
	INTERIOR UNIT BEARING WALL		ADDITIONAL LOADING SEE SHEET S0.1

WOOD TRUSS LEGEND (U.N.O.)			
 XXT-XX/XXX ↑ B/C DEAD LOAD (PSF) ↑ T/C DEAD LOAD (PSF)	TYPE	LOADING	HATCH
	RESIDENTIAL	18T-40Z05	NONE
	PUBLIC AREA	18T-100Z05	
	LIGHT STORAGE/ MED.	18T-125Z05	

NOTES:  
1. AREAS ON PLAN INDICATE DIFFERENCES IN TRUSS DEPTH OR LOADING. SEE PLAN FOR TRUSS DESIGN CRITERIA.

DISCLAIMER NOTES

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Seal

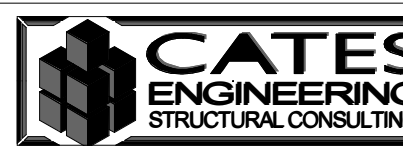


**MOORE & MOORE ARCHITECTURE**  
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CADD File:  
Drawn By: RCR/MRP  
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No. Date Description



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**Daleville Town Center Apartments III**  
an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

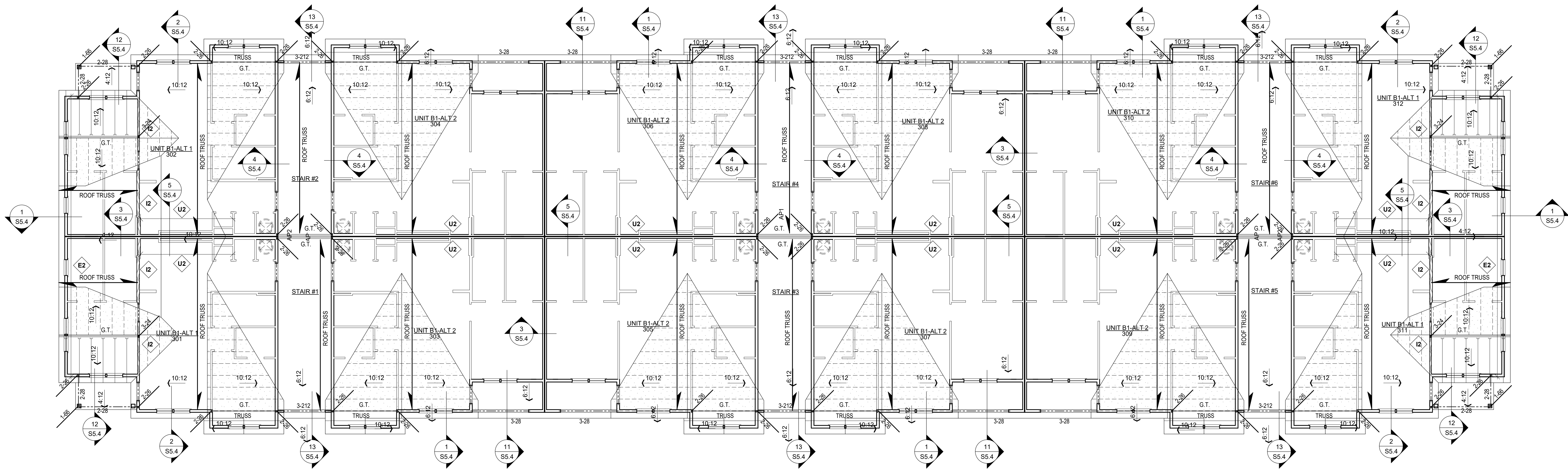
Drawing Title:  
THIRD FLOOR FRAMING  
PLAN - BUILDING 1

**S3.12**

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**ROOF FRAMING PLAN - BUILDING 1**  
SCALE: 1/8" = 1'-0"

- ROOF FRAMING NOTES (WOOD):**
- ALL ROOF FRAMING SHALL BE PRE-ENGINEERED ROOF TRUSSES DESIGNED AND SPACED BY TRUSS MANUFACTURER AT 24" O.C. MAX FOR THE DESIGN LOADS.
  - ALL ROOF TRUSS BEARING POINTS SHALL BE ATTACHED TO THE BUILDING WITH TOE NAILS PER THE SCHEDULE AND SIMPSON H2.5T HURRICANE CLIPS OR EQUIVALENT. ADDITIONAL HOLD-DOWN ANCHORS MAY BE REQUIRED FOR GIRDER TRUSSES AND WILL BE IDENTIFIED DURING THE SHOP DRAWING REVIEW.
  - HOLD-DOWN ANCHORS ARE REQUIRED ON FLUSH BEAMS SUPPORTING ROOF TRUSSES AND WILL BE IDENTIFIED DURING THE SHOP DRAWING REVIEW. U.N.O. USE EITHER SIMPSON H6 OR TWO SIMPSON H2.5T ANCHORS AT EACH END OF BEAM.
  - SEE ARCHITECTURAL PLANS FOR LOCATION OF DRAFTSTOPPING WHERE REQUIRED.
  - ALL DOUBLE TOP PLATES WITH ROOF TRUSSES BEARING TO BE 5/8".
  - SEE BEARING WALL SCHEDULE ON SHEET S0.2 FOR STUD QUANTITY AND SPACING.
  - TRUSS TO TRUSS CONNECTION TO BE SPECIFIED BY TRUSS MANUFACTURER.
  - SEE ROOF FRAMING DETAILS ON S5.4.

TYPICAL ROOF HEADERS (U.N.O.)			
	LOCATION	MARK	MAXIMUM OPENING
1	EXTERIOR	3-28	5'-6"
2	BREEZEWAY	3-28	3'-3"
3	INTERIOR	2-28	3'-1"

NOTES:  
1. FOR DOUBLE AND TRIPLE WINDOWS, ALL HEADERS ARE CONTINUOUS MULTI-SPAN HEADERS WITH (2) 2x6 POSTS BETWEEN U.N.O.  
2. MAXIMUM OPENING CORRELATES TO ROUGH OPENING DIMENSION.

ROOF FRAMING LEGEND			
	DROPPED 2x HEADER		GIRDER TRUSS
	FLUSH 2x HEADER		TRUSS SPAN DIRECTION
	INTERIOR UNIT BEARING WALL		WALL TYPE
			ROOF OVERBUILT AREAS

**DISCLAIMER NOTES**

A. THE CONTRACTOR, BEFORE STARTING ANY WORK, SHALL CHECK ALL DIMENSIONS GIVEN ON THE STRUCTURAL DRAWINGS, RELATING TO GRID LINES, COLUMN AND WALL LOCATIONS, STRUCTURAL AND FINISHED FLOOR ELEVATIONS, MEMBER SIZES, ETC., WITH THE ARCHITECTURAL AND CIVIL DRAWINGS. IF ANY DISCREPANCIES ARE NOTICED, IMMEDIATELY NOTIFY THE SER/ARCHITECT, AND WORK SHALL NOT COMMENCE UNTIL INSTRUCTIONS ARE RECEIVED FROM THE SER/ARCHITECT.

B. THE CONTRACTOR SHALL REFER TO THE SER/ARCHITECT FOR ANY DIMENSION NOT GIVEN ON OR NOT OBTAINABLE FROM THE CONSTRUCTION DRAWINGS. THE CONTRACTOR SHALL NOT USE SCALE TO OBTAIN OR VERIFY ANY DIMENSION SHOWN ON THESE DRAWINGS.

**Daleville Town Center Apartments III**  
an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:  
ROOF FRAMING PLAN -  
BUILDING 1

**S3.13**

Seal

**ZPA**  
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Project: 681-224-25  
CADD File:  
Drawn By: RCR/MRP  
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Permit Release:  
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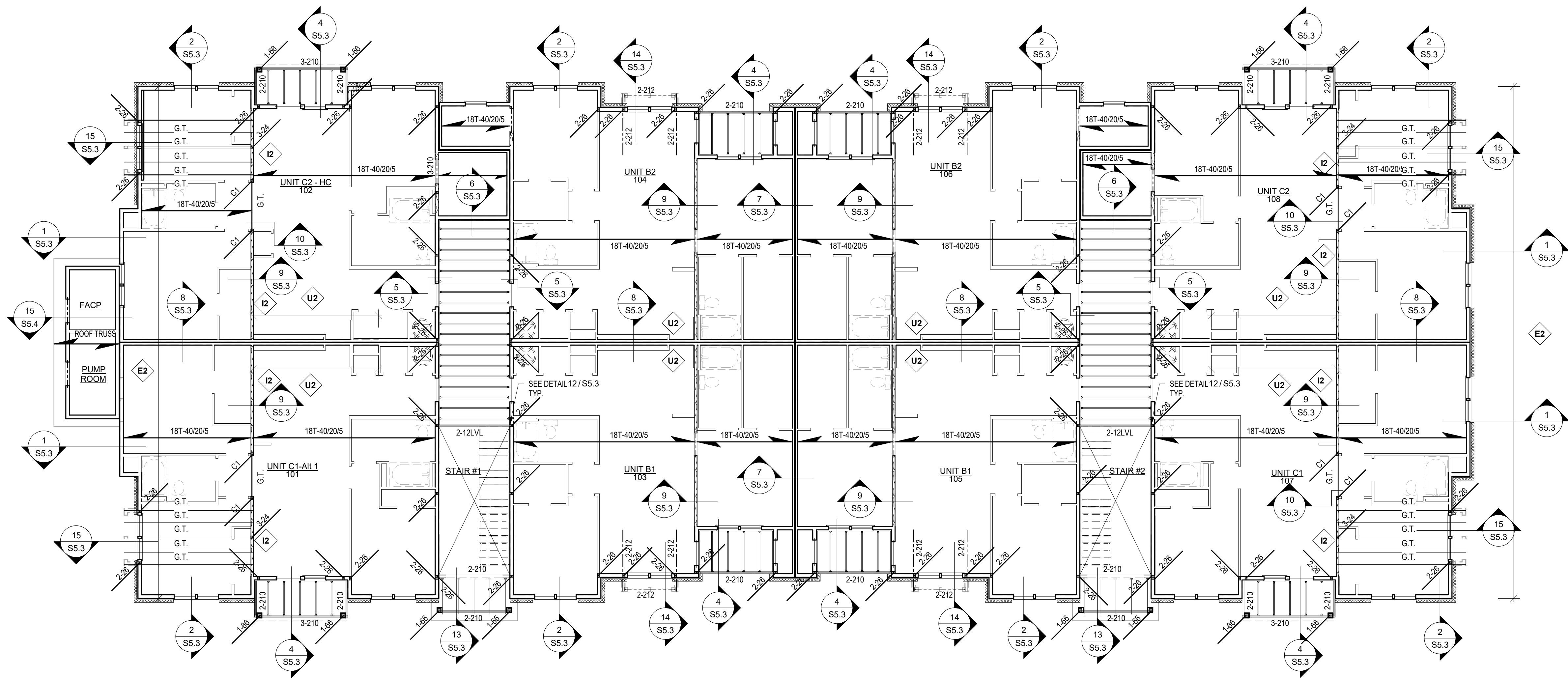
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## SECOND FLOOR FRAMING PLAN - BUILDING 2

SCALE: 1/8" = 1'-0"

### FLOOR FRAMING NOTES (WOOD)

1. ALL FLOOR FRAMING SHALL BE 18" DEEP PRE-ENGINEERED OPEN WEB WOOD TRUSSES DESIGNED AND SPACED BY TRUSS MANUFACTURER AT 24" O.C. MAX. U.N.O. FOR THE DESIGN LOADS.
2. FLOOR TRUSS MANUFACTURER TO PROVIDE BLOCKOUTS FOR MECHANICAL DUCTS. VERIFY SIZE AND LOCATION OF FLOOR OPENINGS WITH LATEST ARCHITECTURAL AND MEP DRAWINGS.
3. ALL TRUSSES TO TRUSS CONNECTIONS TO BE SPECIFIED BY TRUSS MANUFACTURER.
4. EXTERIOR DECK MEMBERS SHALL BE 2" x 12" SPACED MATERIAL. REFER TO BALCONY FRAMING SCHEDULE FOR JOIST AND LEDGER SIZE AND ATTACHMENT.
5. PROVIDE (1) 2x6 STRONGBACK AT APPROXIMATE MID-SPAN OF THE TRUSSES UNDER 20'-0" OR APPROXIMATE THIRD POINTS OF TRUSSES OVER 20'-0". MAINTAIN 10'-0" MAX. SPACING.
6. FOR STACKING NON BEARING WALLS FRAMED PARALLEL WITH FLOOR TRUSSES, PROVIDE 2x4 CRIPPLE STUDS AT 24" O.C. IN FLOOR PLENUM BETWEEN WALLS. SEE DETAIL S1/S5.1.
7. SEE BEARING WALL SCHEDULE ON SHEET S0.2 FOR STUD QUANTITY AND SPACING.
8. EXTERIOR WALLS TO BE TYPE "E1" U.N.O., INTERIOR UNIT BEARING WALLS TO BE TYPE "I1" U.N.O., BREEZEWAY WALLS TO BE TYPE "CY" U.N.O., UNIT SEPARATION WALLS TO BE TYPE "U1" U.N.O. AND WET WALLS TO BE TYPE "W1" U.N.O.
9. PRE-ENGINEERED STAIRS BY OTHERS. FABRICATOR TO SUBMIT SHOP DRAWINGS FOR APPROVAL BY ARCHITECT AND ENGINEER.
10. REFER TO S5.1 FOR TYPICAL FRAMING DETAILS.

### TYPICAL FLOOR HEADERS (U.N.O.)

	LOCATION	MARK	MAXIMUM OPENING
1	EXTERIOR		
2	EXTERIOR AT BALCONY	TRUSS HEADER, SEE FRAMING DETAILS	
3	INTERIOR	2x8	3'-1"
4	BREEZEWAY	TRUSS HEADER, SEE FRAMING DETAILS	

NOTES:  
1. FOR DOUBLE AND TRIPLE WINDOWS, ALL HEADERS ARE CONTINUOUS MULTI-SPAN HEADERS WITH (2) 2x6 POSTS BETWEEN U.N.O.  
2. MAXIMUM OPENING CORRELATES TO ROUGH OPENING DIMENSION.

### FLOOR FRAMING LEGEND

	FLUSH TRUSS HEADER		GIRDER TRUSS
	FLUSH 2x4 HEADER		TRUSS SPAN DIRECTION
	DROPPED 2x4 HEADER		WALL TYPE
	INTERIOR UNIT BEARING WALL		ADDITIONAL LOADING SEE SHEET S0.1

### WOOD TRUSS LEGEND (U.N.O.)

	TYPE	LOADING	HATCH
 XXT-XX/XXX ↑ B/C DEAD LOAD (PSF) ↑ T/C DEAD LOAD (PSF)	RESIDENTIAL	18T-40/20/5	NONE
	PUBLIC AREA	18T-100/20/5	
	LIGHT STORAGE/ MECH.	18T-125/20/5	

NOTES:  
1. AREAS ON PLAN INDICATE DIFFERENCES IN TRUSS DEPTH OR LOADING. SEE PLAN FOR TRUSS DESIGN CRITERIA.

### DISCLAIMER NOTES

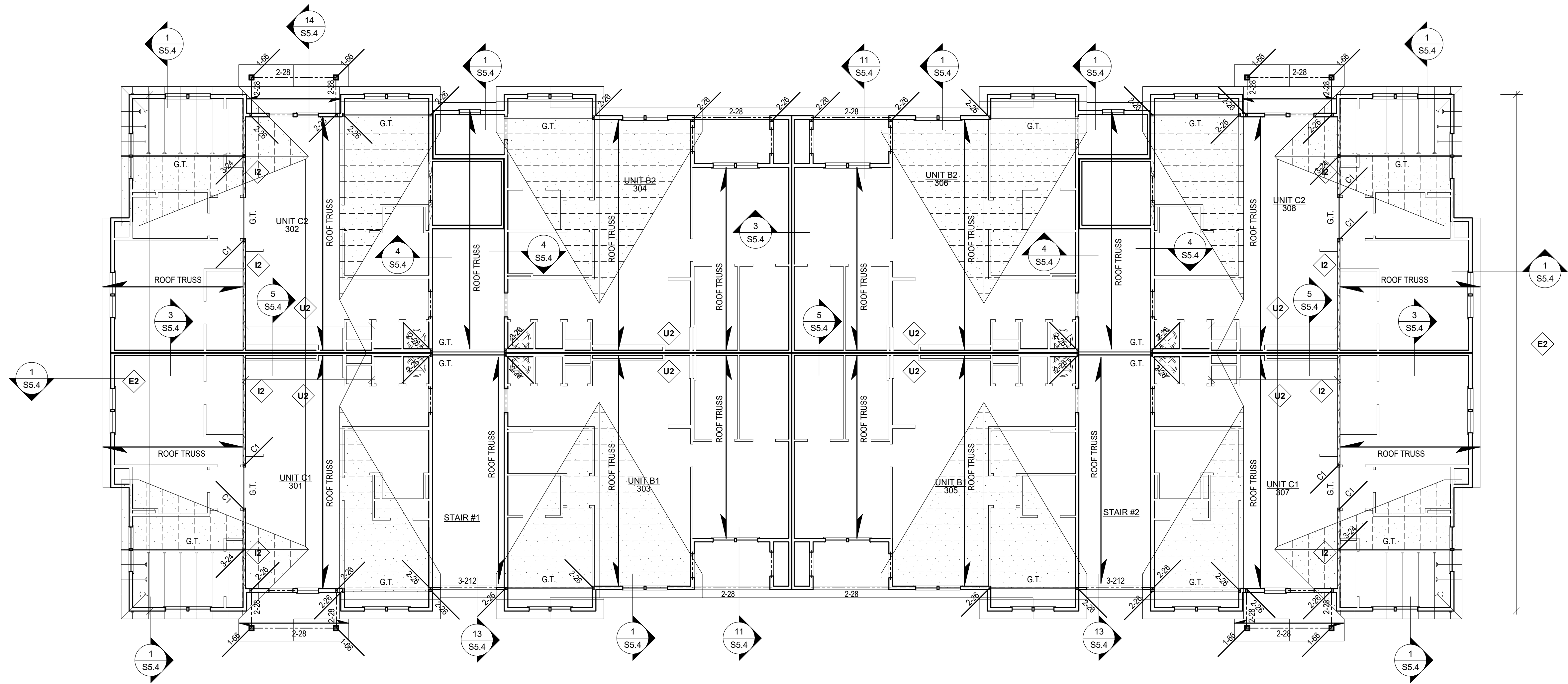
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ROOF FRAMING PLAN - BUILDING 2

SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES (WOOD):

1. ALL ROOF FRAMING SHALL BE PRE-ENGINEERED ROOF TRUSSES DESIGNED AND SPACED BY TRUSS MANUFACTURER AT 24" O.C. MAX FOR THE DESIGN LOADS.
2. ALL ROOF TRUSS BEARING POINTS SHALL BE ATTACHED TO THE BUILDING WITH TOE NAILS PER THE SCHEDULE AND SIMPSON H2.5T HURRICANE CLIPS OR EQUIVALENT. ADDITIONAL HOLDOWN ANCHORS MAY BE REQUIRED FOR GIRDER TRUSSES AND WILL BE IDENTIFIED DURING THE SHOP DRAWING REVIEW.
3. HOLDOWN ANCHORS ARE REQUIRED ON FLUSH BEAMS SUPPORTING ROOF TRUSSES AND WILL BE IDENTIFIED DURING THE SHOP DRAWING REVIEW. U.N.O. USE EITHER SIMPSON H6 OR TWO SIMPSON H2.5T ANCHORS AT EACH END OF BEAM.
4. SEE ARCHITECTURAL PLANS FOR LOCATION OF DRAFTSTOPPING WHERE REQUIRED.
5. ALL DOUBLE TOP PLATES WITH ROOF TRUSSES BEARING TO BE SYPW2.
6. SEE BEARING WALL SCHEDULE ON SHEET S0.2 FOR STUD QUANTITY AND SPACING.
7. TRUSS TO TRUSS CONNECTION TO BE SPECIFIED BY TRUSS MANUFACTURER.
8. SEE ROOF FRAMING DETAILS ON S5.4

TYPICAL ROOF HEADERS (U.N.O.)

	LOCATION	MARK	MAXIMUM OPENING
1	EXTERIOR	3-28	5'-6"
2	BREEZEWAY	3-28	3'-3"
3	INTERIOR	2-28	3'-1"

NOTES:

1. FOR DOUBLE AND TRIPLE WINDOWS, ALL HEADERS ARE CONTINUOUS MULTI-SPAN HEADERS WITH (2) 2x6 POSTS BETWEEN U.N.O.
2. MAXIMUM OPENING CORRELATES TO ROUGH OPENING DIMENSION.

ROOF FRAMING LEGEND

	DROPPED 2x HEADER		GIRDER TRUSS
	FLUSH 2x HEADER		TRUSS SPAN DIRECTION
	INTERIOR UNIT BEARING WALL		WALL TYPE
			ROOF OVERBUILT AREAS

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Daleville Town Center Apartments III

an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:

ROOF FRAMING PLAN -  
BUILDING 2

S3.23

Seal



**POOLE & POOLE ARCHITECTURE**  
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Project: 681-224-25

CADD File:

Drawn By: RCR/MRP

Checked By: MRP

Permit Release:

February 19, 2026

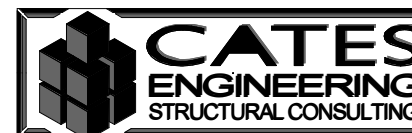
Construction Release Set:

Revisions

No.	Date	Description
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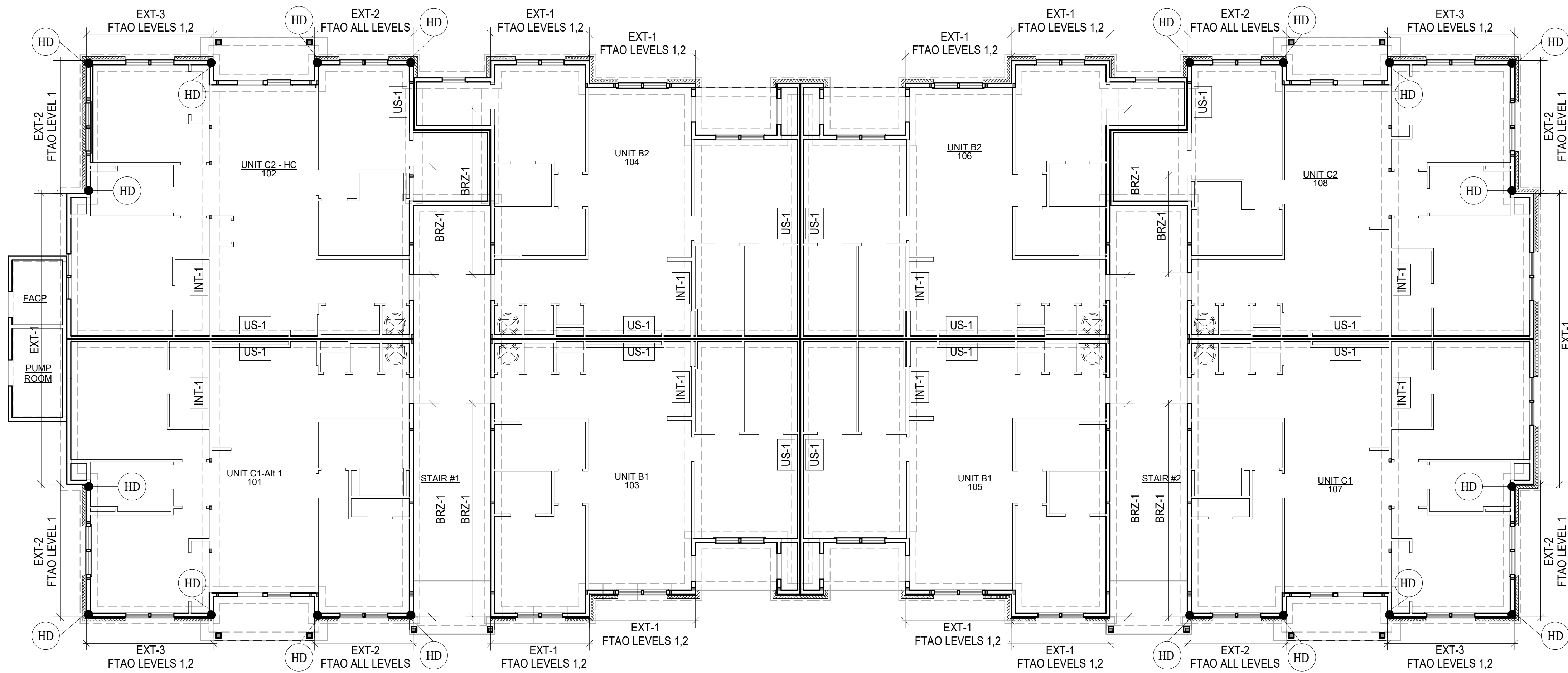
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SHEAR WALL LAYOUT PLAN - BUILDING 2

SCALE: 1/8" = 1'-0"

SHEAR WALL PLAN NOTES (WOOD):

1. EXT-X, INT-X, US-X, COR-X REFERS TO SHEAR WALL DESIGNATION ON LAYOUT PLAN. WALLS WITHOUT DESIGNATION HAVE NOT BEEN DESIGNED AS LATERAL FORCE RESISTING ELEMENTS.
2. EACH "HD" OR "AR" CALLOUT REPRESENTS A HOLDDOWN OR ANCHOR ROD LOCATION, RESPECTIVELY AT UNIT SEPARATION WALLS WHERE TWO "HD" CALLOUTS ARE INDICATED FOR ONE LOCATION, INSTALL (1) HOLDDOWN AT EACH OF THE TWO UNIT SEPARATION WALLS.
3. "FTAO" INDICATES WALL DESIGNED FOR FORCE TRANSFER AROUND OPENINGS METHOD PER NATIONAL DESIGN SPECIFICATION.

**DISCLAIMER NOTES**

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Daleville Town Center Apartments III

an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:  
SHEAR WALL LAYOUT  
PLAN - BUILDING 2

S4.2

Seal



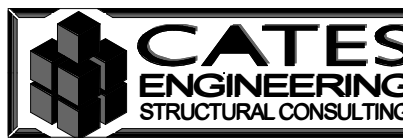
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Project: 681-224-25  
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No. Date Description

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3 STORY SHEAR WALL CHORD AND HOLDDOWN SCHEDULE																			
LEVEL	WOOD SETTLEMENT	EXTERIOR EXT-1		EXTERIOR EXT-2		EXTERIOR EXT-3		UNIT SEPARATION US-1		BREEZEWAY BRZ-1		INTERIOR INT-1							
ROOF																			
3	1-1/8"	1.6k	0.0k	1.3k	0.0k	2.7k	-1.2k	0.8k	0.0k	1.0k	0.0k								
FLOOR		--	N/A	--	N/A	--	S1	--	N/A	--	N/A								
2	3/4"	3.9k	0.0k	3.8k	-1.0k	2.7k	-1.2k	1.8k	0.0k	2.8k	0.0k								
FLOOR		--	N/A	--	S1	--	S1	--	N/A	--	N/A								
1	3/8"	6.4k	ASD = 0.0k LRFD = 0.0k	5.6k	ASD = -1.2k LRFD = -2.6k	3.4k	ASD = -1.2k LRFD = -2.0k	3.2k	ASD = 0.0k LRFD = 0.0k	5.0k	ASD = 0.0k LRFD = 0.0k	2.7k	ASD = 0.0k LRFD = 0.0k						
SLAB		--	N/A	--	HD-1	--	HD-1	--	N/A	--	N/A								

STRAP AND HOLDDOWN LEGEND

CUMULATIVE COMPRESSION (k)

CUMULATIVE TENSION (k)

CHORD STUDS E.E. OF SHEARWALL (1)

DENOTES STRAP

DENOTES HOLDDOWN

STRAP # AT WOOD FLRS.

HOLDDOWN # AT CONCRETE FLRS.

SYMBOL ON SHEARWALL LAYOUT PLAN: ● HD

STRAPS

NUMBER	TYPE	NAILING	MAX LOAD	END LENGTHS
S1	CS18	(22) 0.131"x2-1/2" NAILS	1370 lbs	12 in
S2	CS14	(30) 0.148"x2-1/2" NAILS	2490 lbs	16 in
S3	CMSTC16	(58) 0.148"x3-1/4" NAILS	4690 lbs	25 in

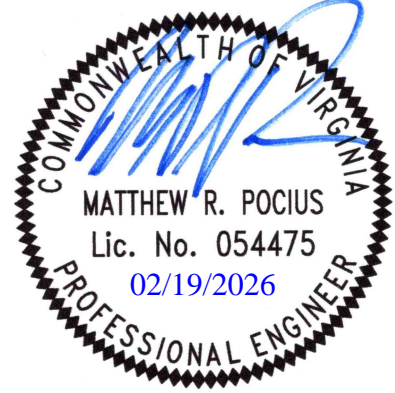
HOLDDOWNS

NUMBER	TYPE	FASTENING	HOLDDOWN MAX LOAD	ANCHOR BOLT
HD-1	LTPP2	(12) 0.148"x1-1/2" NAILS	1965 lbs	5/8"x10 ALL THREAD ROD W/ 6" EMBED SET-3G EPOXY, STANDARD CUT WASHER AT LTPP2
HD-2	HTT4	(18) 0.148"x1-1/2" NAILS	3105 lbs	
HD-3	HTT5	(26) 0.148"x2-3" NAILS	4015 lbs	
HD-4	HDIUE9-S DS3.5	(16) SDS 1/4"x3-1/2" SCREWS	7995 lbs	7/8"x10 ALL THREAD ROD W/ 9" EMBED SET-3G EPOXY

1. CHORD STUDS TO BE TYPICAL BUILT UP POSTS LOCATED EACH END OF SHEARWALL. NUMBER OF STUDS SHOWN IS THE **TOTAL STUD** REQUIREMENT EACH END OF WALL INCLUDING STUDS INSTALLED PER THE TYPICAL STUD WALL CORNER AND INTERSECTION DETAIL. IF CHORD STUD CELL IS BLANK, NO ADDITIONAL STUDS ARE REQUIRED BEYOND THOSE STUDS INSTALLED PER THE TYPICAL STUD WALL CORNER AND INTERSECTION DETAILS.  
2. CHORD STUD REQUIREMENTS AT UNIT SEPARATION WALLS REFER TO (1) WALL OF THE (2) WALL ASSEMBLY. REFER TO SHEARWALL LAYOUT PLAN FOR CHORD STUD REQUIREMENTS AT SPECIFIC SHEARWALL DESIGNATIONS.  
3. AT ANCHOR ROD CONDITIONS, REFER TO STUD WALL CORNER AND INTERSECTION DETAIL FOR STUD PLACEMENT AROUND ANCHOR ROD.  
4. AT LOCATIONS WHERE STRAPS ARE CALLED OUT, BOTTOM OF THE STRAPS SHALL BE ONLY ATTACHED AT THE END OF THE FRAMING PROCESS JUST PRIOR TO INSTALLING DRYWALL.  
5. ALL HEX NUTS AT ANCHOR ROD BEARING PLATES TO BE SNUG TIGHTENED AT THE END OF THE FRAMING PROCESS JUST PRIOR TO INSTALLING DRYWALL.  
6. NUMBER OF STRAPS CALLED OUT ARE TO BE INSTALLED AT EACH END OF THE WALL, U.N.O.  
7. AT ALL EXTERIOR WALLS WHERE VERTICAL STRAPS ARE CALLED OUT, STRAPS SHALL BE INSTALLED ON THE INSIDE FACE OF THE WALL.  
8. ALL HOLDDOWNS, ANCHORS, AND STRAPS TO BE "SIMPSON STRONG TIE" PRODUCTS, U.N.O. CONTRACTOR MAY SUBMIT ALTERNATE FOR APPROVAL.

STUD WALL SHEATHING SCHEDULE - 3 STORY												
WALL LOCATION	WALL LEVEL			WALL FACE	SHEATHING TYPE CODE	SHEATHING(1)	FASTENER SIZE		SPACING EDGE FIELD	WALL BLOCKING	SHEAR (7) CAPACITY	
	1	2	3				SIZE					
EXTERIOR WALLS WHERE NOTED ON SHEARWALL LAYOUT PLAN (1,4,5)												
EXT-1 EXT-2 EXT-3	● ● ●			OUTER	OSB612	7/16" OSB	OUTER: MIN. GALV. 0.113"x2-3/8" NAILS		8"	12"	336 PLF	
				INNER	GW6812UNB	5/8" GWB			8"	12"	UNLOCKED 70 PLF	
				OUTER	OSB412	7/16" OSB			4"	12"	BLOCKED 490 PLF	
				INNER	GW6812UNB	5/8" GWB			8"	12"	UNLOCKED 70 PLF	
EXT-3				OUTER	OSB312	7/16" OSB	INNER: 1-1/4" TYPE W #6 SCREWS (2)		3"	12"	BLOCKED 630 PLF	
				INNER	GW6812UNB	5/8" GWB			8"	12"	UNLOCKED 70 PLF	
EXTERIOR WALLS (ALL OTHERS)												
TYPICAL	● ● ●			OUTER	OSB612	7/16" OSB	SEE ABOVE		8"	12"	BLOCKED 336 PLF	
				INNER	GW6812UNB	5/8" GWB			8"	12"	UNLOCKED 70 PLF	
BREEZEWAY WALLS WHERE NOTED ON SHEARWALL LAYOUT PLAN (5)												
BRZ-1	● ● ●			EACH SIDE	GW6812UNB	5/8" GWB	1-1/4" TYPE W #6 SCREWS (2)		8"	12"	UNLOCKED 70 PLF	
					GW6812UNB				7"	7"	UNLOCKED 115 PLF	
					GW6812UNB				4"	4"	UNLOCKED 145 PLF	
					GW6812UNB				4"	4"	BLOCKED 175 PLF	
BREEZEWAY WALLS (ALL OTHERS)												
TYPICAL	● ● ●			UNIT SIDE	GW6812UNB	5/8" GWB	SEE ABOVE		8"	12"	UNLOCKED 70 PLF	
UNIT SEPARATION WALLS WHERE NOTED ON SHEARWALL LAYOUT PLAN (5)												
US-1	● ● ●			UNIT SIDE	GW6812UNB	5/8" GWB	1-1/4" TYPE W #6 SCREWS (2)		8"	12"	UNLOCKED 70 PLF	
					GW6812UNB				7"	7"	UNLOCKED 115 PLF	
					GW6812UNB				4"	4"	UNLOCKED 145 PLF	
					GW6812UNB				4"	4"	BLOCKED 175 PLF	
INTERIOR BEARING WALLS WHERE NOTED ON SHEARWALL LAYOUT PLAN (5)												
INT-1	● ● ●			EACH SIDE	GW6812UNB	5/8" GWB	1-1/4" TYPE W #6 SCREWS (2)		8"	12"	UNLOCKED 70 PLF	
					GW6812UNB				7"	7"	UNLOCKED 115 PLF	
					GW6812UNB				4"	4"	UNLOCKED 145 PLF	
					GW6812UNB				4"	4"	BLOCKED 175 PLF	
INTERIOR BEARING WALLS (ALL OTHERS)												
TYPICAL	● ● ●			EACH SIDE	GW6812UNB	5/8" GWB	SEE ABOVE		8"	12"	UNLOCKED 70 PLF	
WOOD BASE PLATE ATTACHMENT SCHEDULE												
WALL LOCATION	CONCRETE ATTACHMENT				WOOD ATTACHMENT							
	OPTION #1		OPTION #2		FASTENER		SPACING		FASTENER		SPACING	
EXTERIOR WALLS												
EXT-1 EXT-2 EXT-3	1/2"x46"	SIMPSON TITEN HD ANCHOR WITH 3x3x0.225" PLATE WASHER	3'-0" O.C.	SIMPSON MASA STRAP WITH STANDARD INSTALLATION, 6" MAX. FROM WALL ENDS OR CORNERS	3'-0" O.C.	3'-0" O.C.		3"x0.131"x10 NAIL		4" O.C.		
EXT-2												
EXT-3												
TYPICAL EXTERIOR	1/2"x46"	SIMPSON TITEN HD ANCHOR	3'-0" O.C.		3'-0" O.C.					4" O.C.		
INTERIOR WALLS												
BRZ-1			8" O.C.	1/2"x46" SIMPSON TITEN HD ANCHOR, 12" MAX. FROM WALL ENDS OR CORNERS	4'-6" O.C.	3"x0.131"x10 NAIL				8" O.C.		
TYPICAL BREEZEWAY	HILTI X-CP72P8S23 POWER ACTUATED FASTENER w/ 20MM WASHERS, 6" MAX. FROM WALL ENDS OR CORNERS	16" O.C.			5'-0" O.C.							
US-1		12" O.C.			5'-0" O.C.							
INT-1		16" O.C.			5'-0" O.C.					8" O.C.		
TYPICAL INT. BRG.		16" O.C.			5'-0" O.C.					8" O.C.		
1. REFER TO FRAMING DETAIL SHEETS FOR STRAP REQUIREMENTS FROM FLOOR TO FLOOR AND BEARING WALL STUD SCHEDULE FOR LOCATIONS OF EDGE BLOCKING IN WALLS. 2. ALTERNATE ATTACHMENTS FOR INTERIOR SHEAR WALLS ONLY: 0.120" NAIL, MIN. 3/8" HEAD, 1-3/4" LONG, OR 16 GAGE STAPLES, 1-1/2" LEGS, 1-5/8" LONG, OR WALLBOARD NAIL (0.0915"x1-7/8", 19/64" HEAD). 3. ● INDICATES NAILING PATTERN TO BE APPLIED AT THAT FLOOR LEVEL. 4. ALL EXTERIOR SHEAR WALLS ARE DESIGNED AS PERFORATED WALLS PER NDS, U.N.O. ON SHEARWALL LAYOUT PLAN. INTERIOR GWB ON EXTERIOR WALLS SHALL NOT BE CONSIDERED IN SHEARWALL CAPACITY FOR SEISMIC DESIGN. 5. SEE SHEARWALL LAYOUT PLAN FOR SHEAR WALL LOCATIONS. 6. SEE SHEARWALL CHORD AND HOLDDOWN SCHEDULE FOR SHEARWALL HOLDDOWN AND CHORD STUD INFORMATION. 7. VALUES ARE PROVIDED PER FACE OF WALL. WOOD STRUCTURAL PANEL SHEAR CAPACITY TO BE DIVIDED BY 1.4 FOR SEISMIC DESIGN. 8. "GWB" TO BE ATTACHED TO ALL INTERMEDIATE FRAMING MEMBERS BETWEEN PANEL EDGES.												

Seal



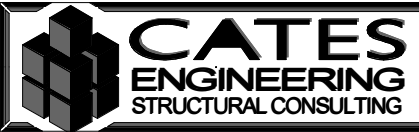
Project: 681-224-225  
CADD File:  
Drawn By: RCR/MRP  
Checked By: MRP  
Permit Release:  
February 19, 2026  
Construction Release Set:

Revisions

No.	Date	Description
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ASI / RFI Revisions

No.	Date	Description
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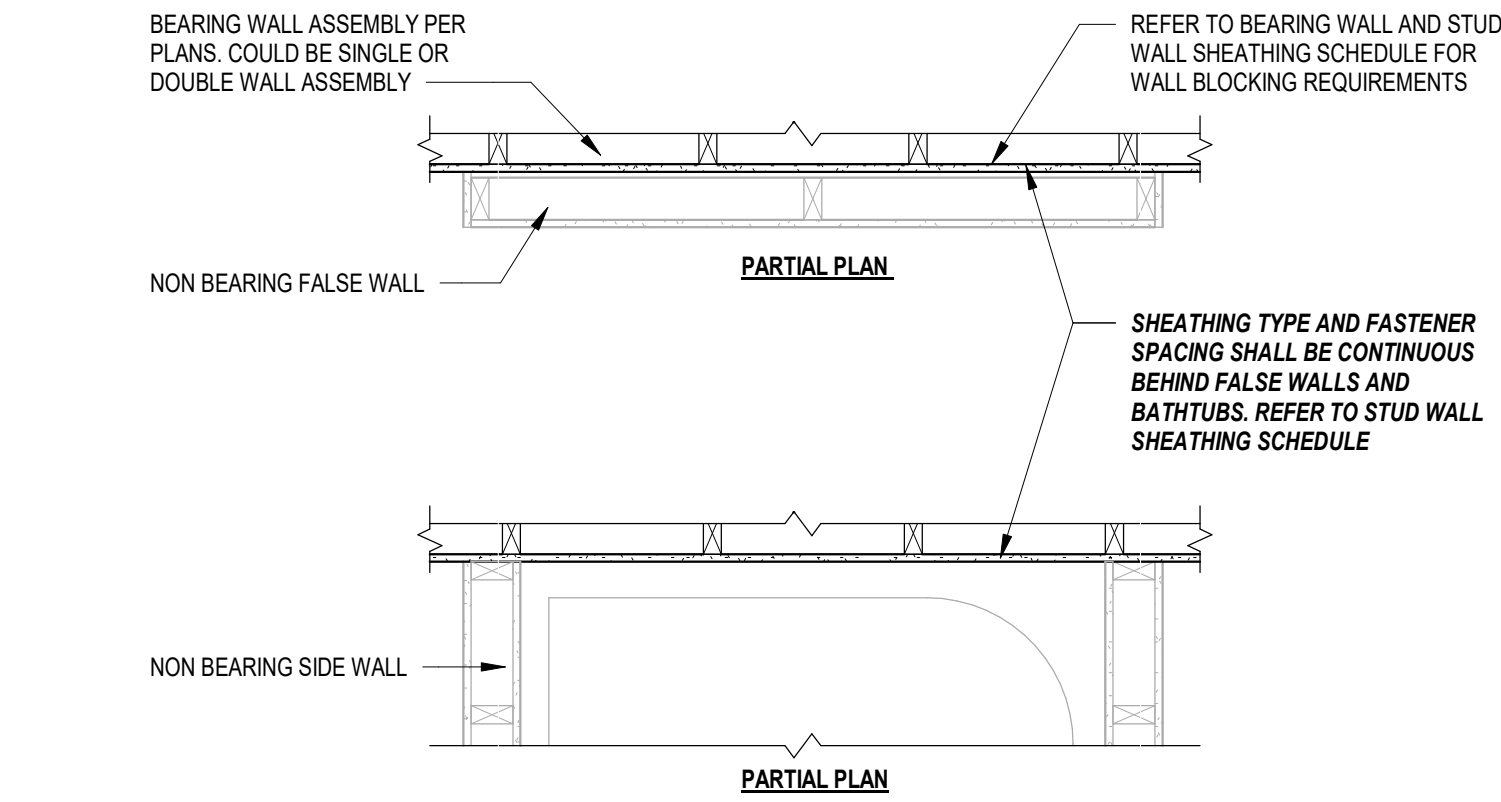
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an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:  
SHEAR WALL SCHEDULES

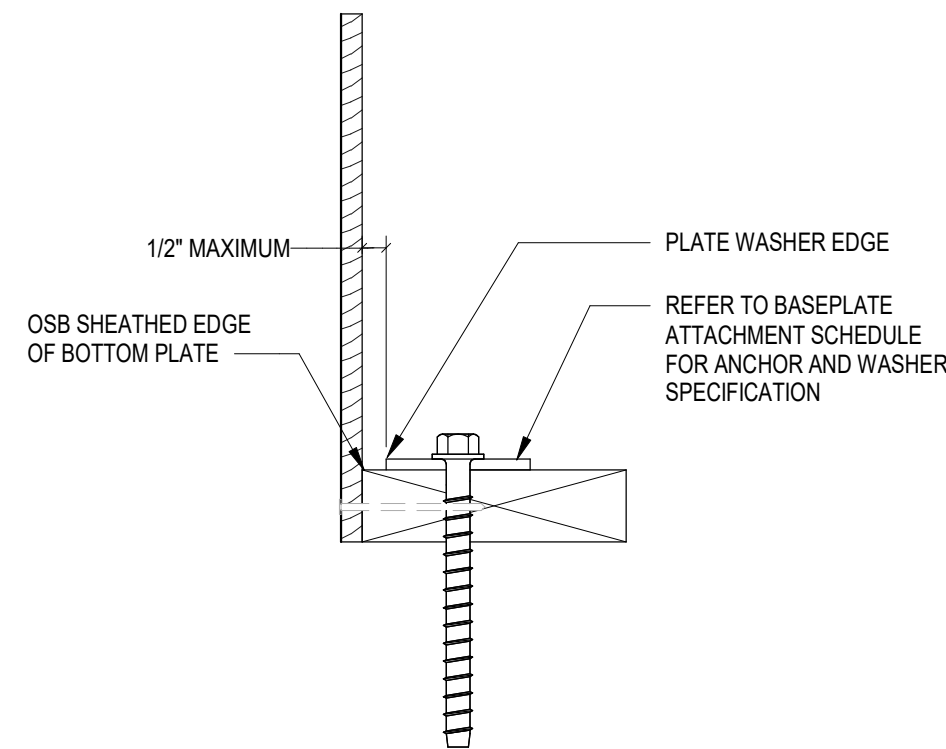
S4.3



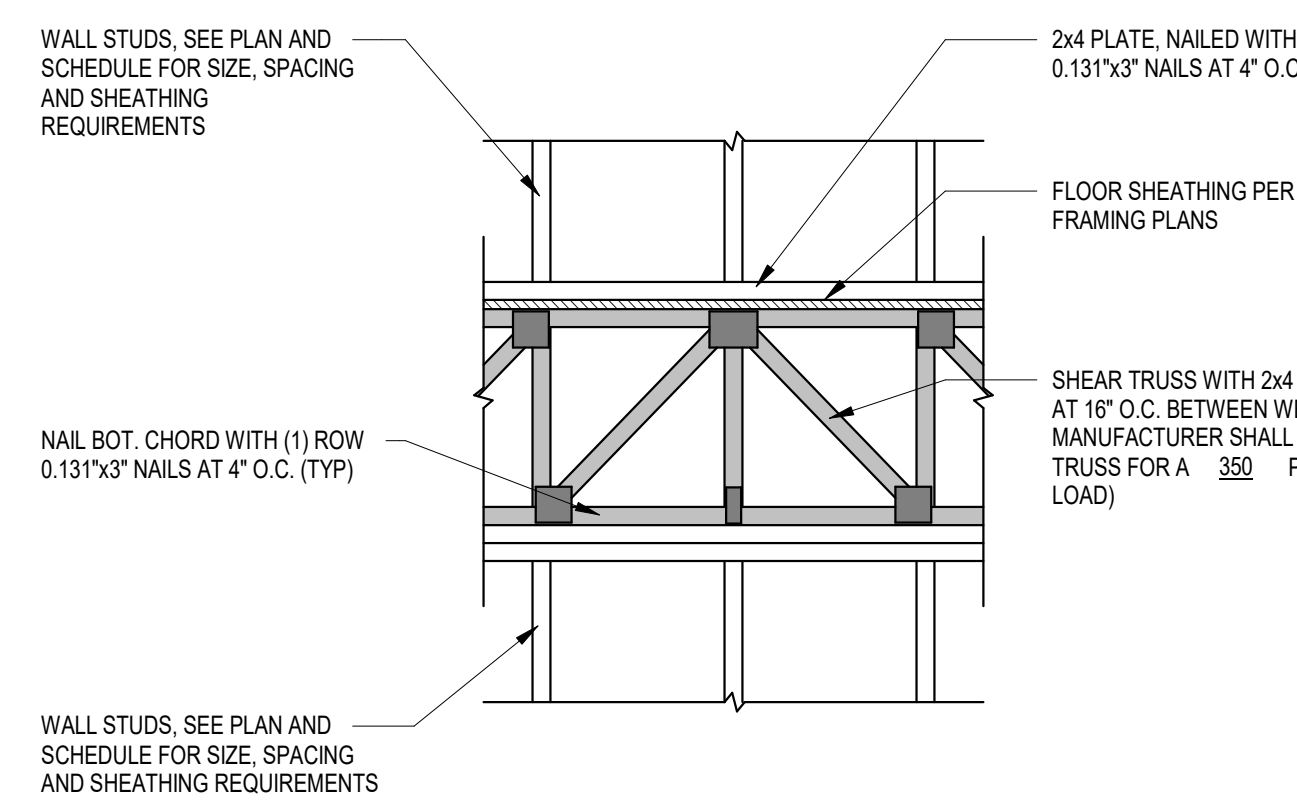
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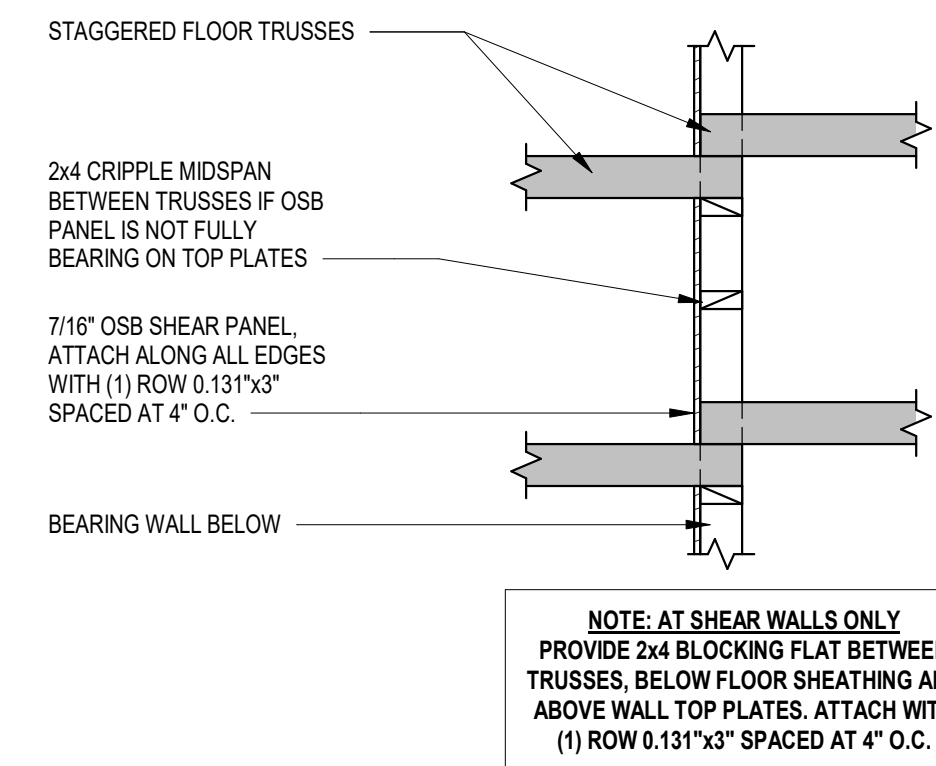
**8**  
**S4.4** **SHEATHING ATTACHMENT BEHIND FALSE WALLS / TUBS**  
SCALE: N.T.S. 12.11.24



**9**  
**S4.4** **DISTANCE FROM WASHER TO SHEATHING EDGE**  
3\"/>

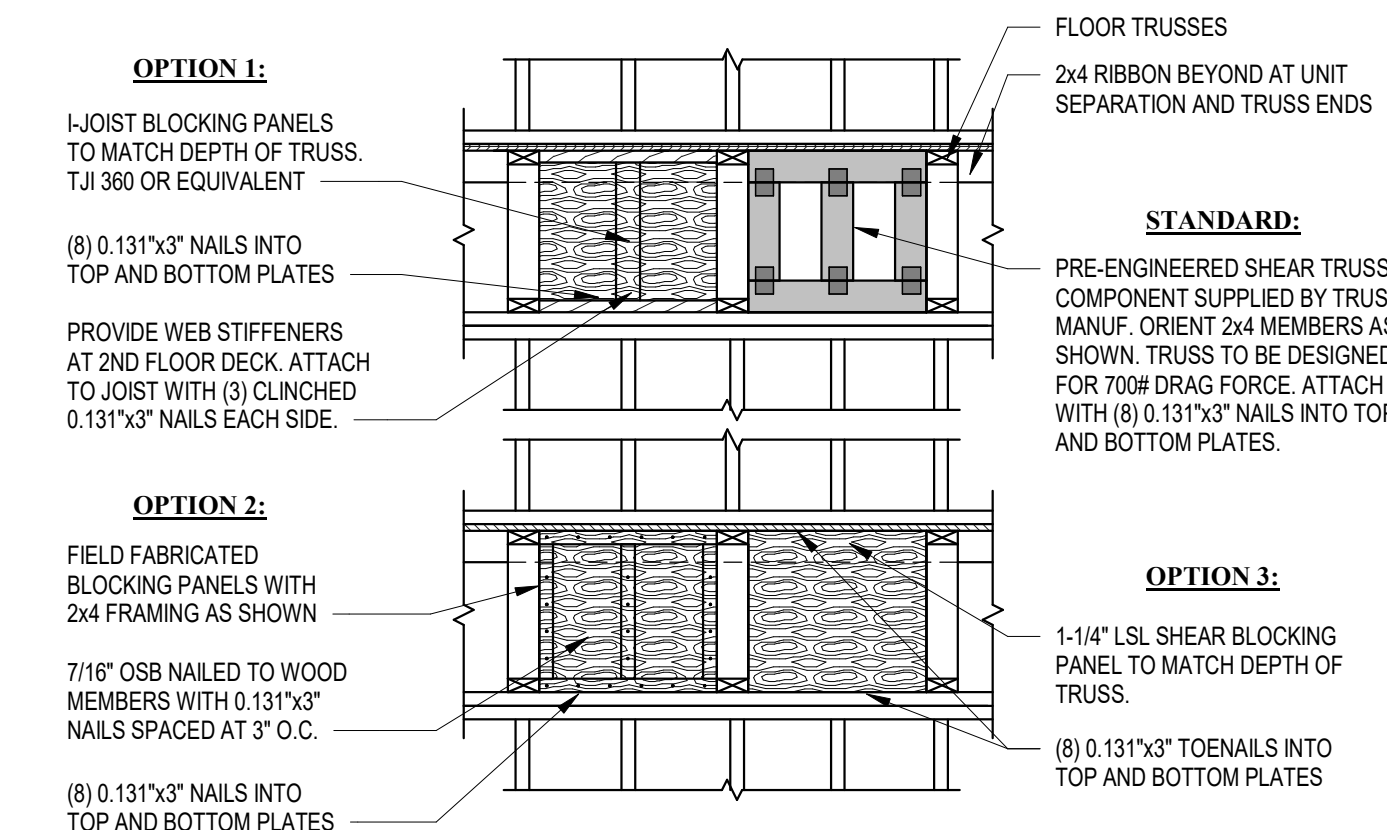


**4**  
**S4.4** **CONTINUOUS BEARING SHEAR TRUSS**  
SCALE: N.T.S.

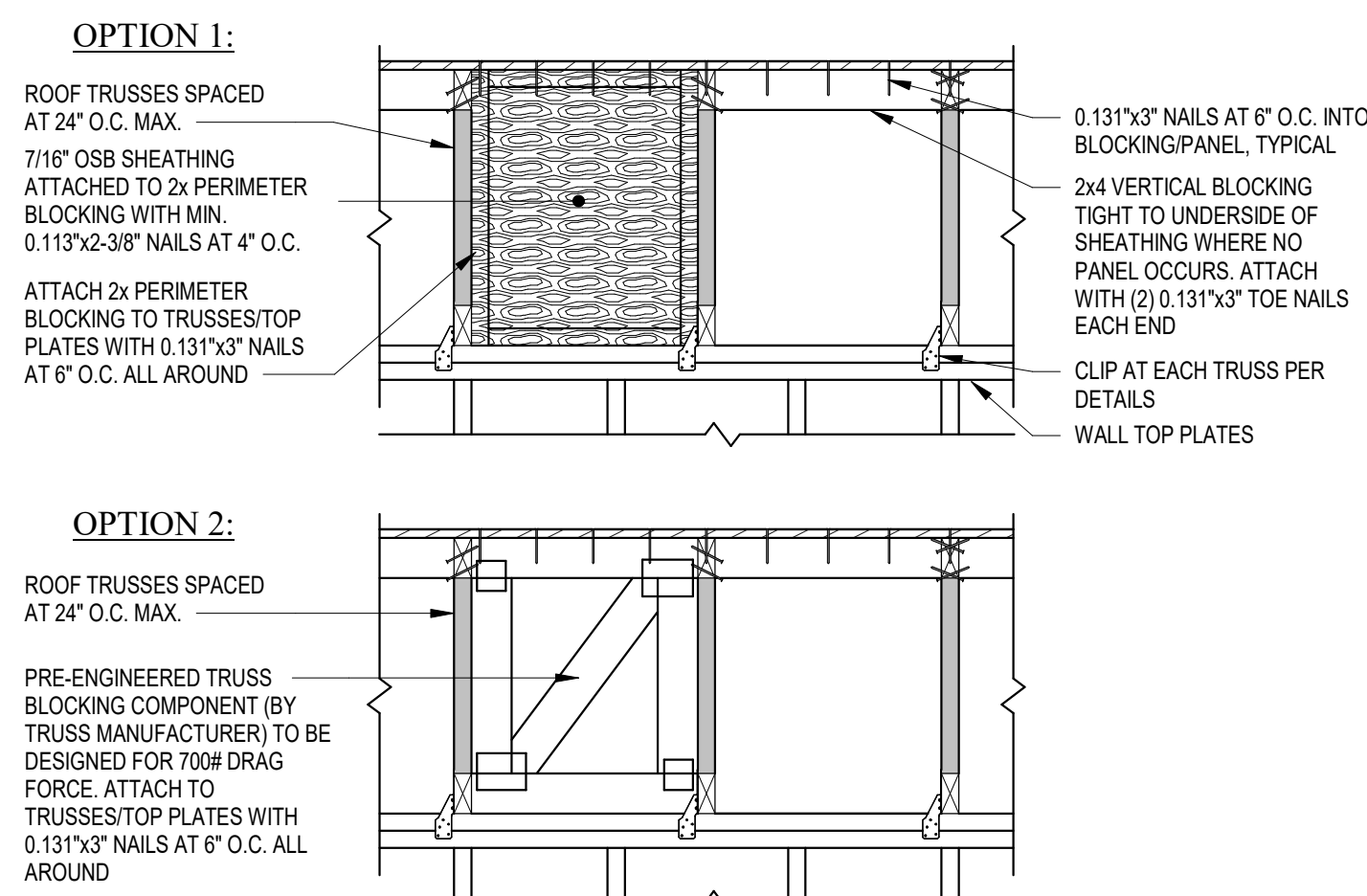


**5**  
**S4.4** **STAGGERED TRUSS BEARING**  
SCALE: N.T.S. 08.06.16

SHEAR BLOCKING SCHEDULE	
	PANEL SPACING AT INTERIOR BEARING WALLS
ROOF LEVEL	NONE
3rd FL. DECK	NONE
2nd FL. DECK	72\"/>

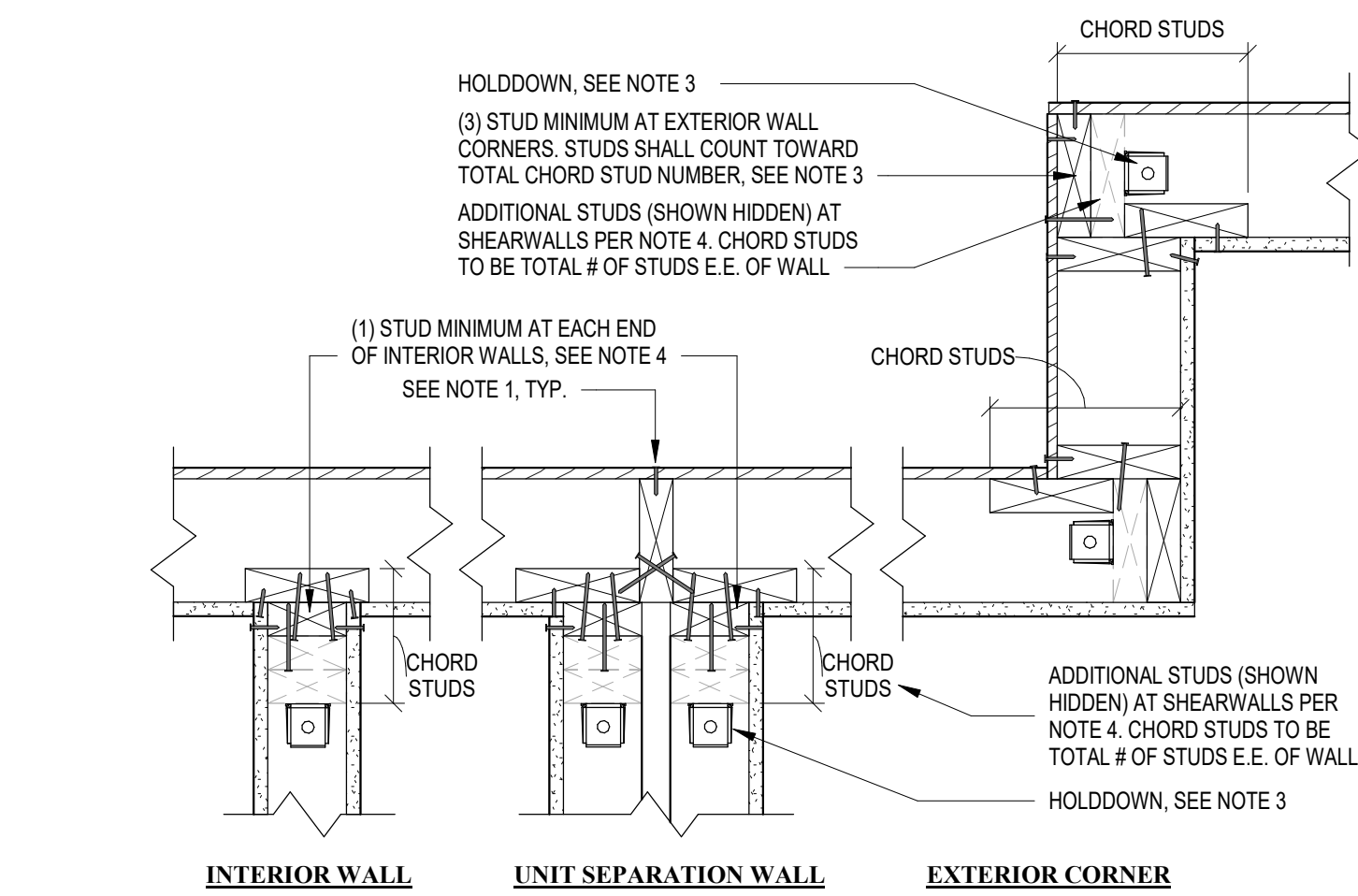


**6**  
**S4.4** **TYPICAL SHEAR BLOCKING AT FLOOR**  
3/4\"/>



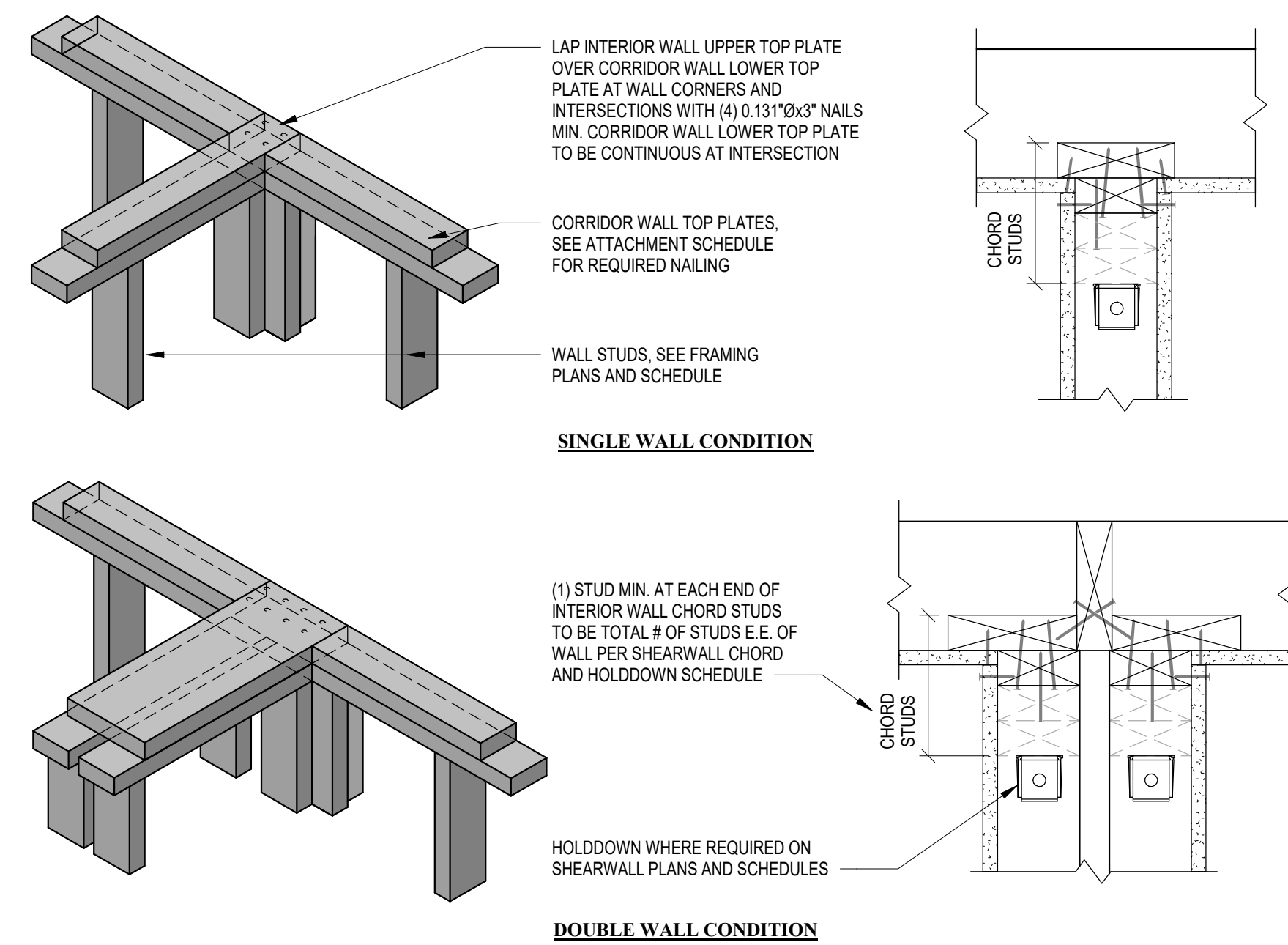
NOTE: SEE SHEAR WALL LAYOUT PLAN AND SHEAR BLOCKING DETAILS FOR SPACING REQUIREMENTS

**7**  
**S4.4** **TYPICAL SHEAR BLOCKING AT ROOF**  
SCALE: N.T.S. WHERE ROOF TRUSSES BEAR ON WALL

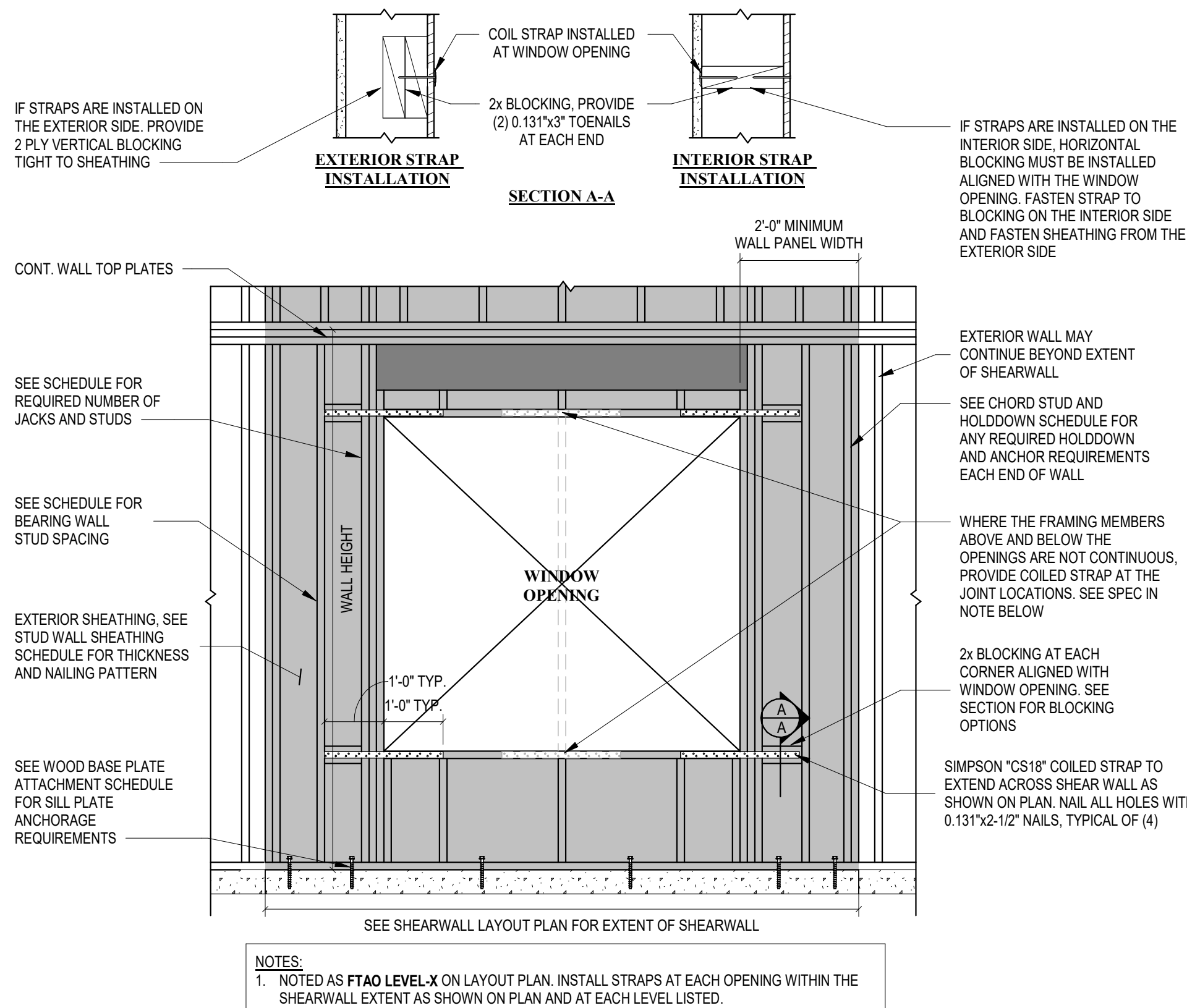


- NOTES:
1. ATTACH EXTERIOR AND INTERIOR SHEATHING TO STUDS PER PLANS AND SCHEDULES
  2. NAIL ALL BUILT-UP STUDS WITH 0.131\"/>

**1**  
**S4.4** **STUD WALL CORNER AND INTERSECTION FRAMING**  
SCALE: N.T.S. AT HOLDDOWN CONDITIONS - TYPE SA 01.06.21



**2**  
**S4.4** **TYPICAL INTERIOR SHEAR WALL INTERSECTION**  
SCALE: N.T.S.



**3**  
**S4.4** **FORCE TRANSFER AROUND OPENING**  
SCALE: N.T.S. 10.31.23

Seal

**Matthew R. Pocus**  
Lic. No. 054475  
02/19/2026  
PROFESSIONAL ENGINEER

**ZPA**

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Project: 681-224-25  
CADD File:  
Drawn By: RCR/MRP  
Checked By: MRP  
Permit Release:  
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Construction Release Set:  
-

Revisions  
No. Date Description

ASI / RFI Revisions  
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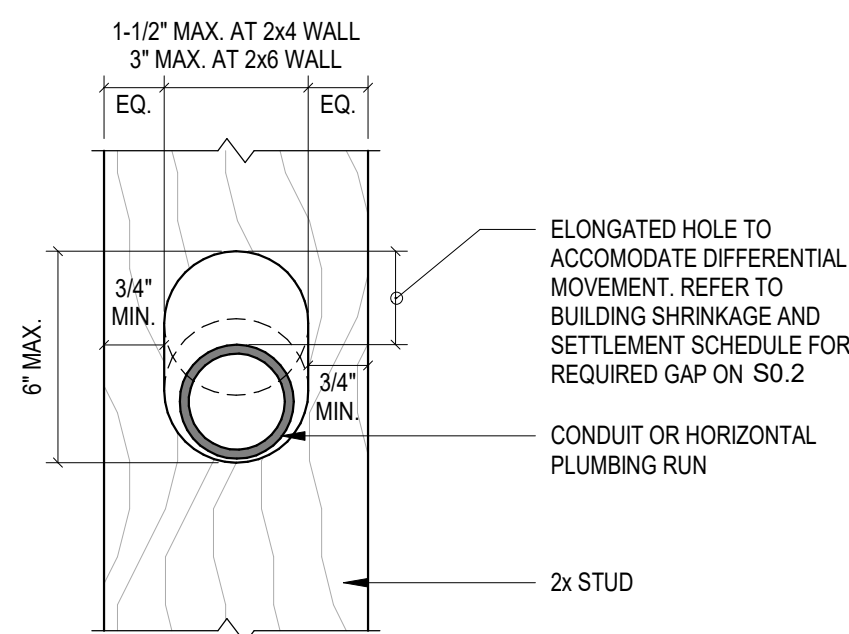
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Drawing Title:  
SHEAR WALL DETAILS

**S4.4**

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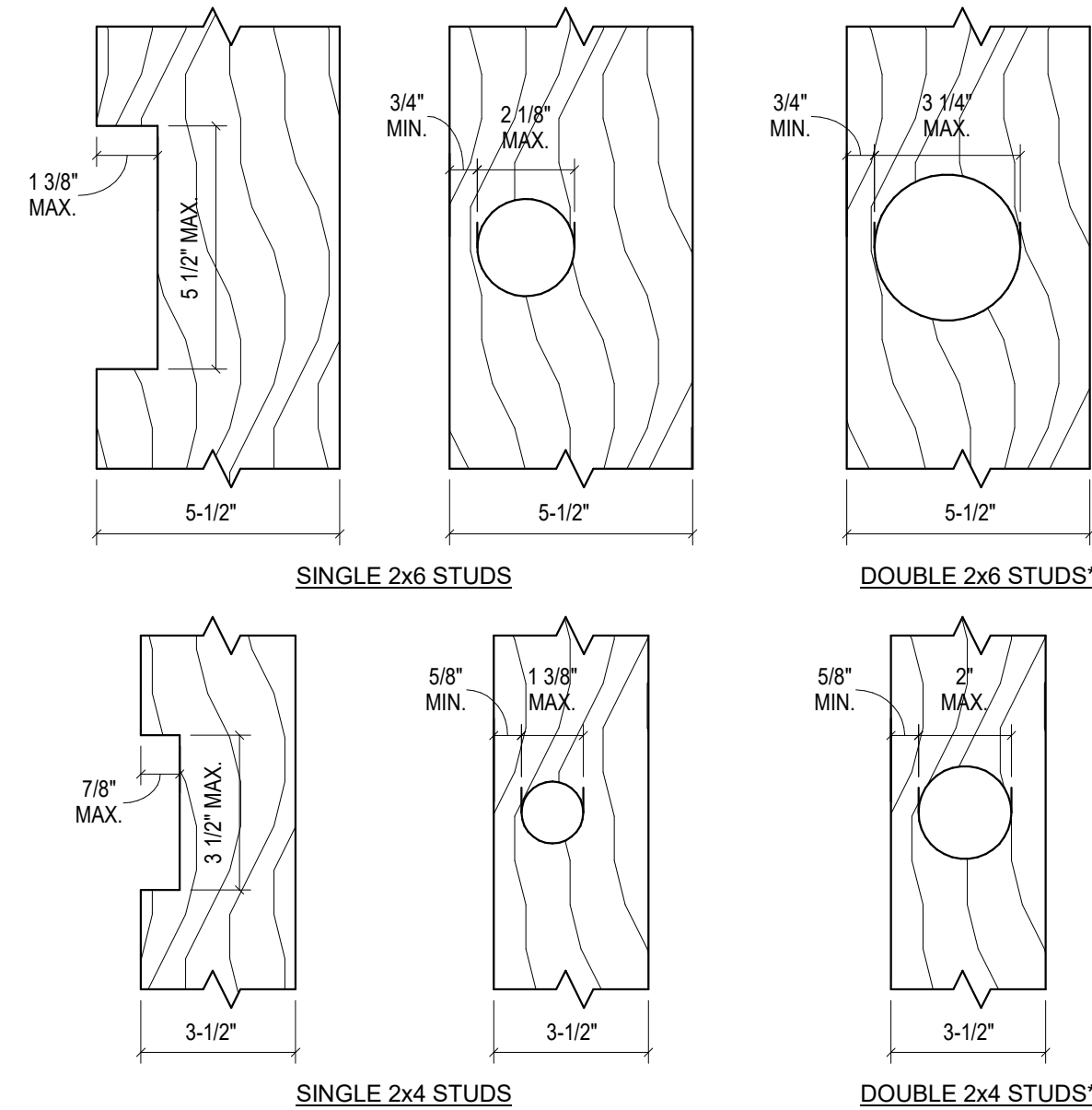


NOTES:  
1. ALLOWABLE HOLE SIZES SHOWN PERMIT FULL AXIAL CAPACITY OF STUD

### 13 S5.1 ELONGATED HOLE DETAIL

SCALE: N.T.S.

11.05.17



#### INTERNATIONAL BUILDING CODE

##### SECTION 2303.9.10 CUTTING AND NOTCHING

IN EXTERIOR WALLS AND BEARING PARTITIONS, ANY WOOD STUD IS PERMITTED TO BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25 PERCENT OF ITS WIDTH. CUTTING OR NOTCHING OF STUDS TO A DEPTH NOT GREATER THAN 40 PERCENT OF THE WIDTH OF THE STUD IS PERMITTED IN NONBEARING PARTITIONS SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE PARTITION.

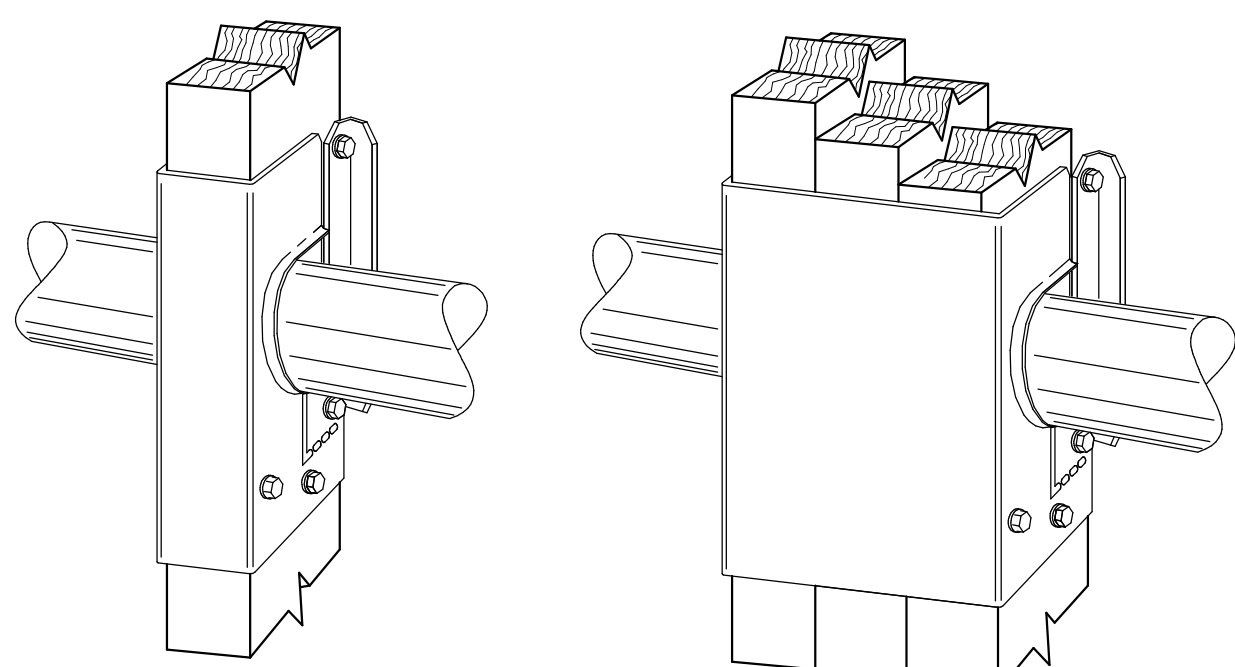
##### SECTION 2303.9.11 BORED HOLES

A HOLE NOT GREATER IN DIAMETER THAN 40 PERCENT OF THE STUD WIDTH IS PERMITTED TO BE BORED IN ANY WOOD STUD. BORED HOLES NOT GREATER THAN 80 PERCENT OF THE WIDTH OF THE STUD ARE PERMITTED IN ANY WALL WHERE EACH STUD IS DOUBLED, PROVIDED NOT MORE THAN TWO SUCH SUCCESSIVE DOUBLED STUDS ARE SO BORED.  
-IN NO CASE SHALL THE EDGE OF THE BORED HOLE BE NEARER THAN 3/8 INCH TO THE EDGE OF THE STUD.  
-BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF STUD AS A CUT OR NOTCH.

### 10 S5.1 ALLOWABLE STUD NOTCH AND BORINGS

SCALE: N.T.S.

STUDS	SIMPSON MODEL NUMBER	FASTENERS
2x	HSS2-SDW51	12-SDS 1/4"x1-1/2"
(2) 2x	HSS2-2-SDW53	12-SDS 1/4"x3"
(3) 2x	HSS2-3-SDW53	12-SDS 1/4"x3"



#### NOTES:

- SS STUD SHOES MAY BE ACCEPTABLE IN LIMITED CIRCUMSTANCES. CONTACT SER FOR EVALUATION PRIOR TO INSTALLATION.
- STUD SHOES ARE NEEDED WHERE LIMITS OF STUD BORING ARE EXCEEDED.
- SHOES SHALL BE INSTALLED PER MANUFACTURER'S GUIDELINES.
- IF MORE THAN 2 ADJACENT STUDS ARE OVERBORED, CONTACT SER FOR EVALUATION.
- FOR USE AT BOTTOM TWO LEVELS OF FRAMING ONLY.

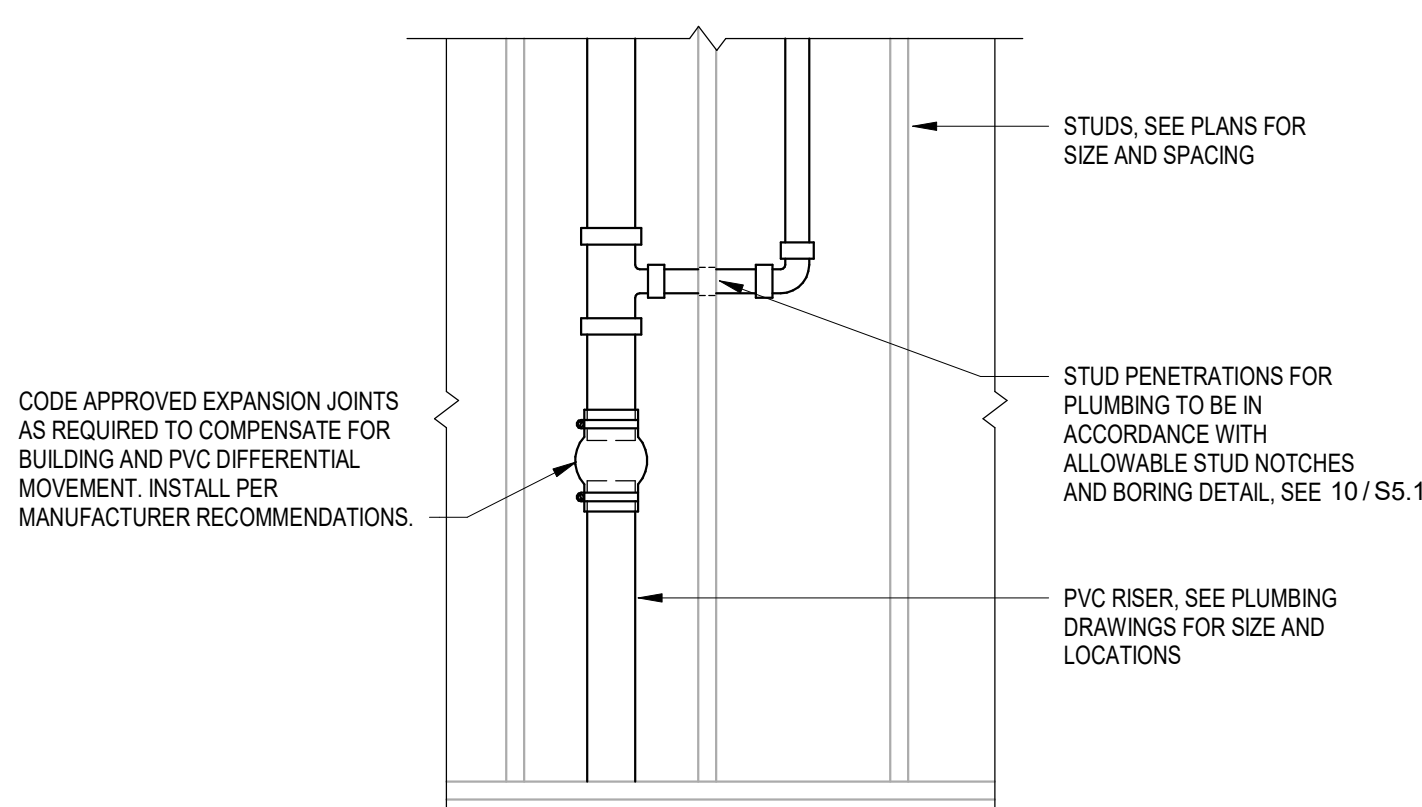
### 11 S5.1 STRUCTURAL REPAIR FOR OVERBORED STUDS

SCALE: N.T.S.

02.01.18

PVC THERMAL MOVEMENT SCHEDULE		
FRAMED LEVEL	REFERENCE ELEVATION (FT.)	EXPANSION/ CONTRACTION (IN.)
ROOF	52.1	1.27
4TH FLOOR	43.1	1.05
3RD	32.4	0.79
2ND	21.4	0.52
1ST	10.7	0.26

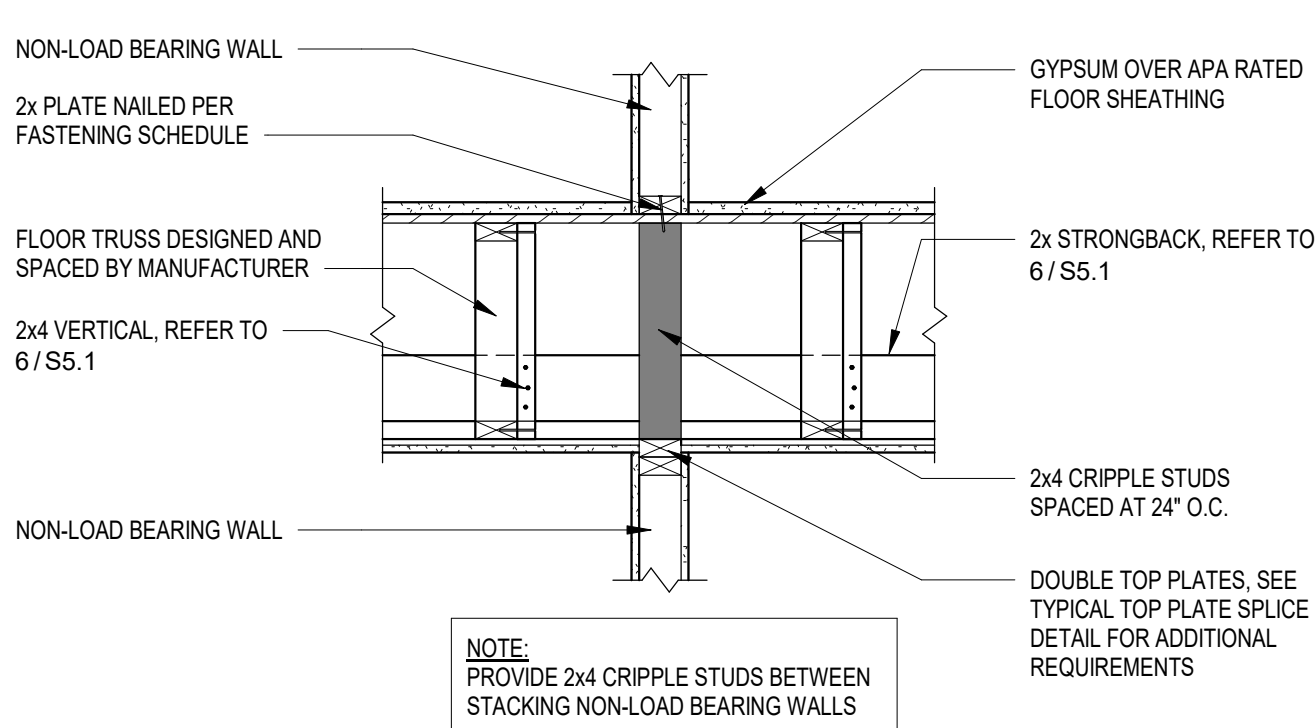
NOTES:  
1. VALUES BASED ON 70 DEGREE TEMPERATURE DIFFERENCE  
2. VALUES BASED ON COEFFICIENT OF THERMAL EXPANSION OF 2.9e-5in./in./F



### 12 S5.1 PVC PIPE/BLDG SETTLEMENT DETAIL

SCALE: N.T.S.

11.14.22

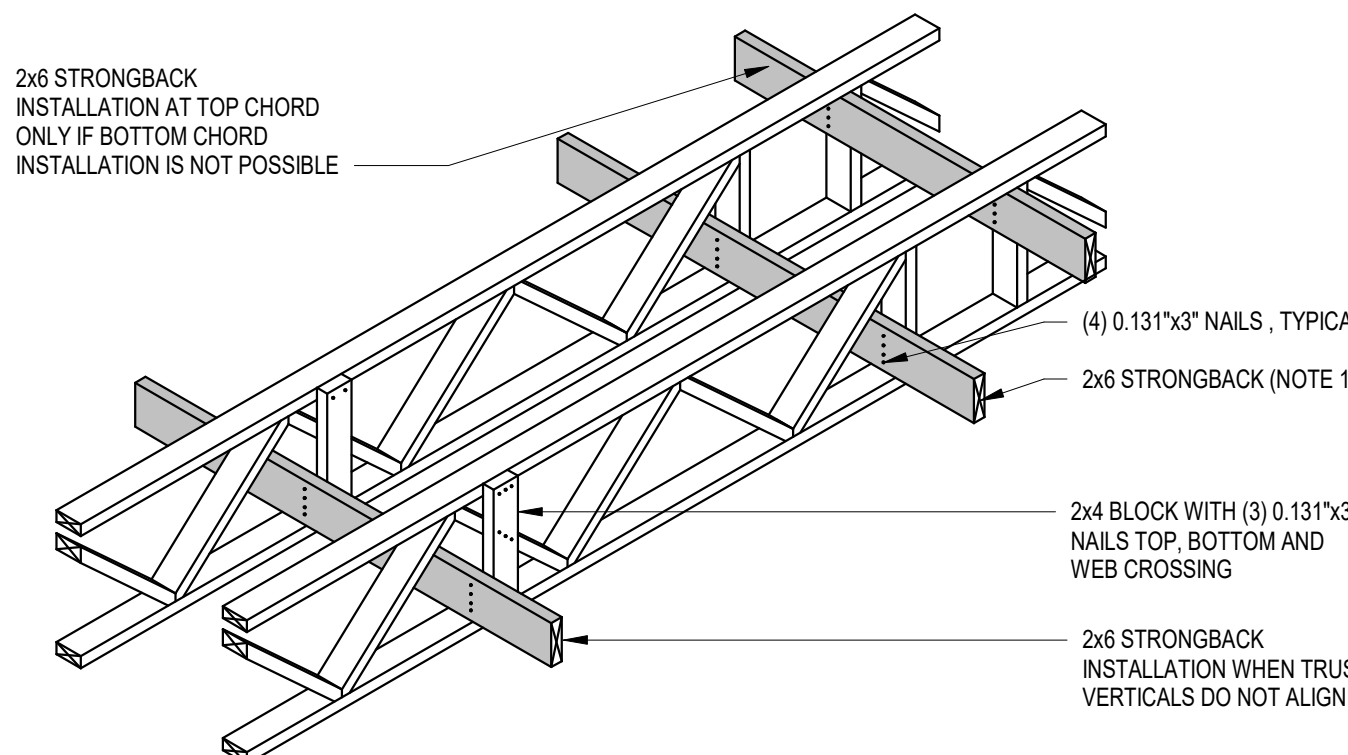


NOTE: PROVIDE 2x4 CRIPPLE STUDS BETWEEN STACKING NON-LOAD BEARING WALLS

### 5 S5.1 NON-BEARING WALL CRIPPLE DETAIL

SCALE: N.T.S.

10.31.23

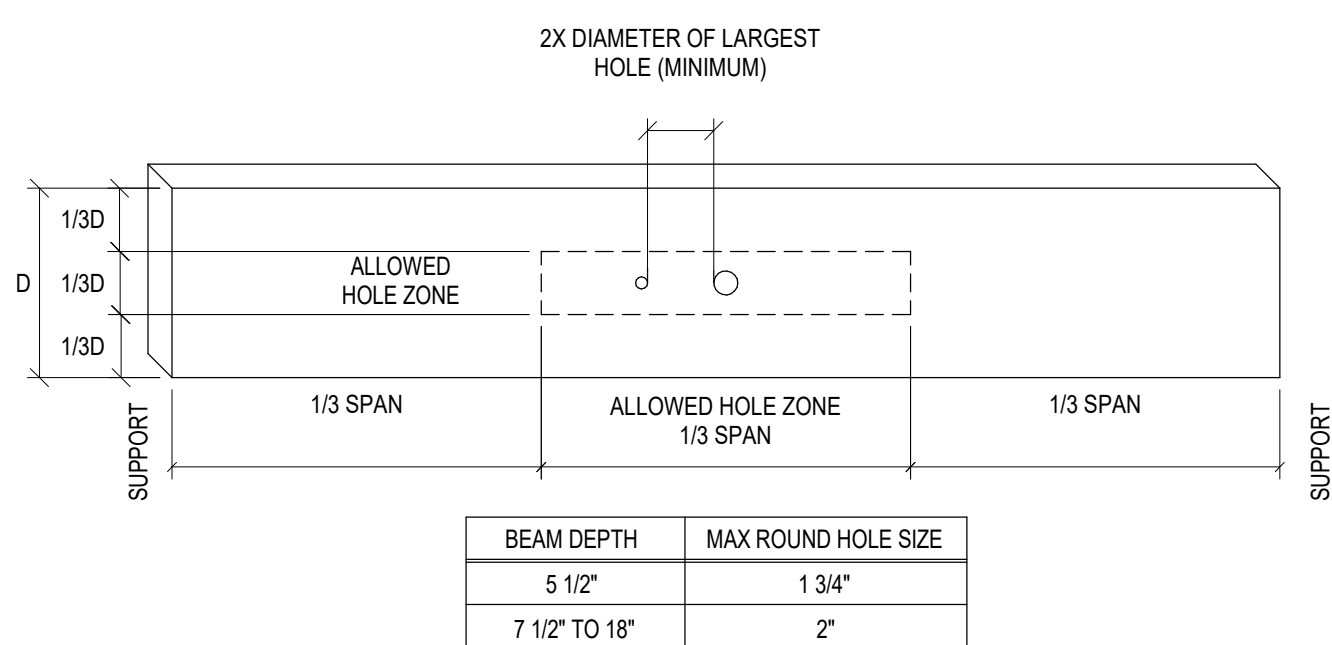


#### NOTES:

- LOCATE STRONGBACKS AT APPROXIMATE MID SPAN OF TRUSSES UNDER 20'-0\"/>

### 6 S5.1 TYPICAL STRONGBACK DETAIL

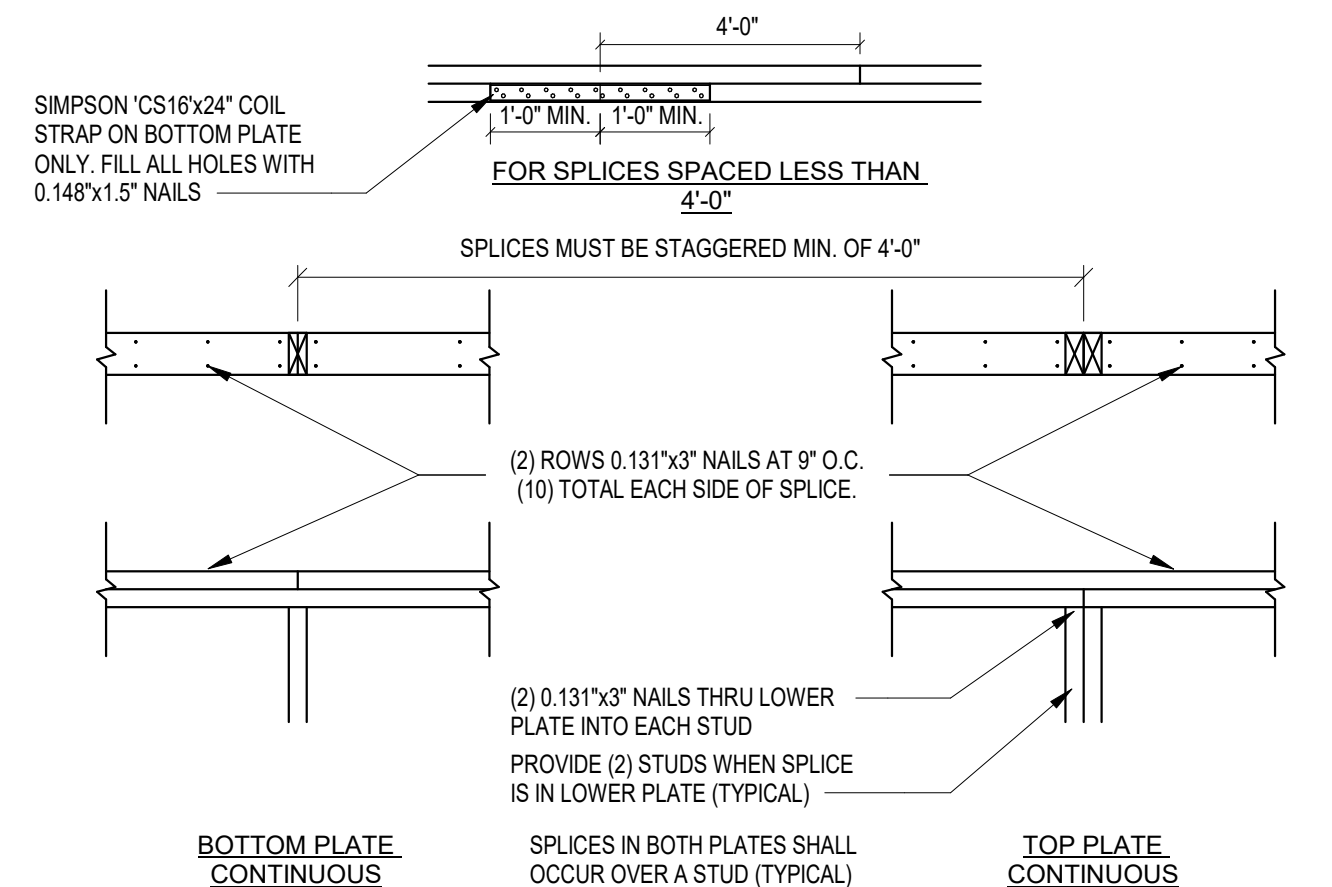
SCALE: N.T.S.



GENERAL NOTES:  
1. DIAGRAM IS FOR UNIFORMLY LOADED BEAM ONLY.  
2. RECTANGULAR HOLES ARE NOT ALLOWED

### 7 S5.1 ALLOWABLE HOLE GUIDE FOR BEAM OR HEADER

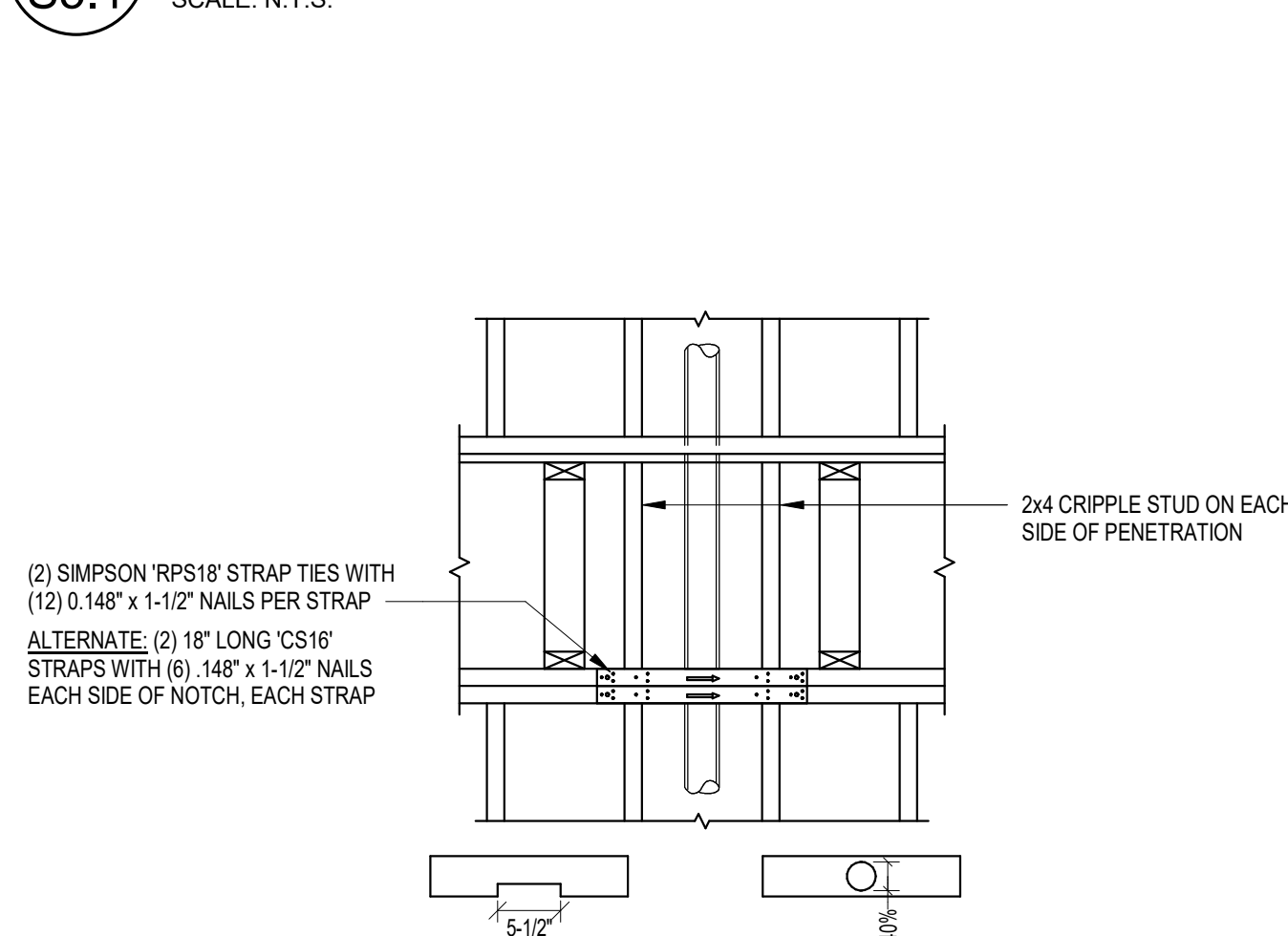
SCALE: N.T.S.



### 8 S5.1 TYPICAL TOP PLATE SPLICE

SCALE: N.T.S.

02.22.17

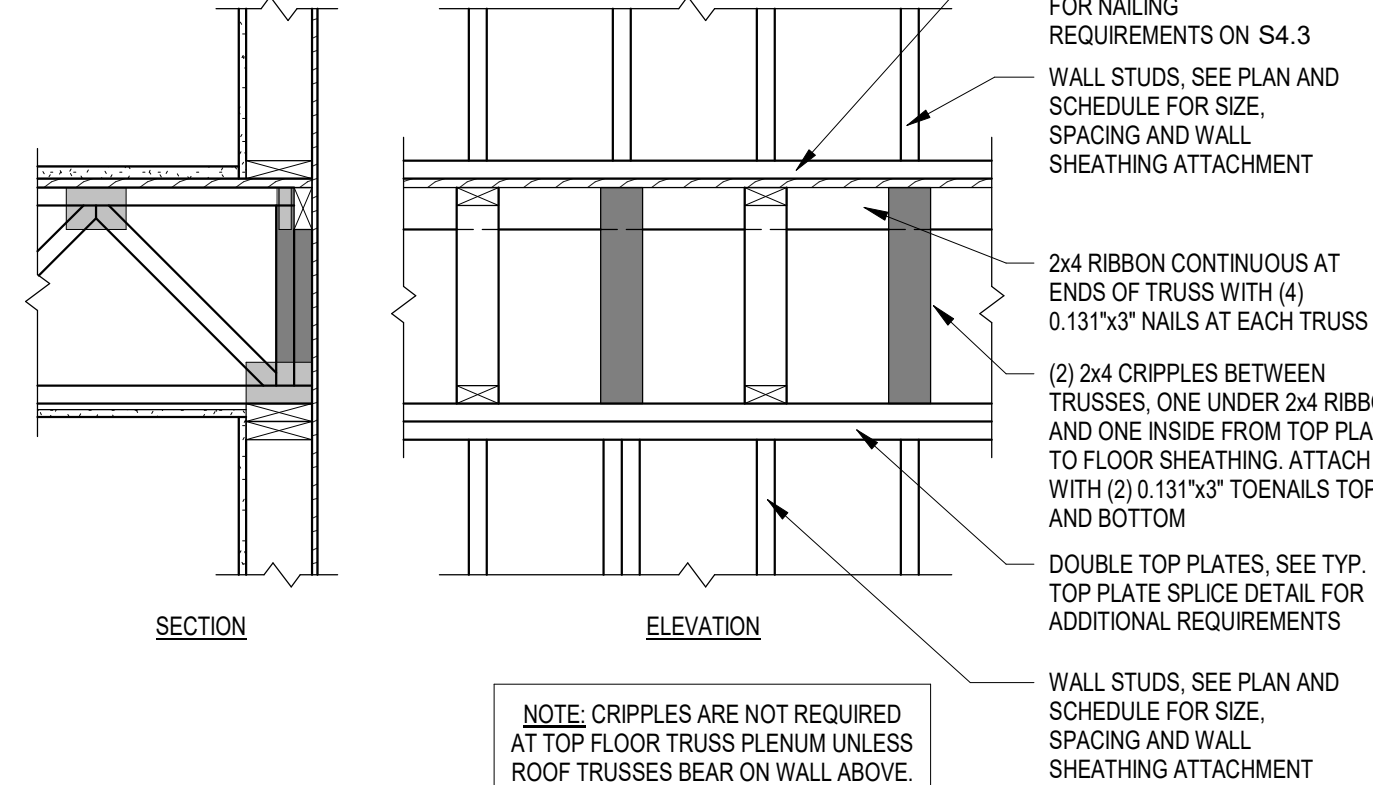


FOR NOTCHES LESS THAN 5-1/2\"/>

### 9 S5.1 TOP PLATE PENETRATIONS

SCALE: N.T.S.

11.21.22



NOTE: CRIPPLES ARE NOT REQUIRED AT TOP FLOOR TRUSS PLENUM UNLESS ROOF TRUSSES BEAR ON WALL ABOVE.

### 1 S5.1 EXTERIOR WALL CRIPPLE AT TRUSS ENDS

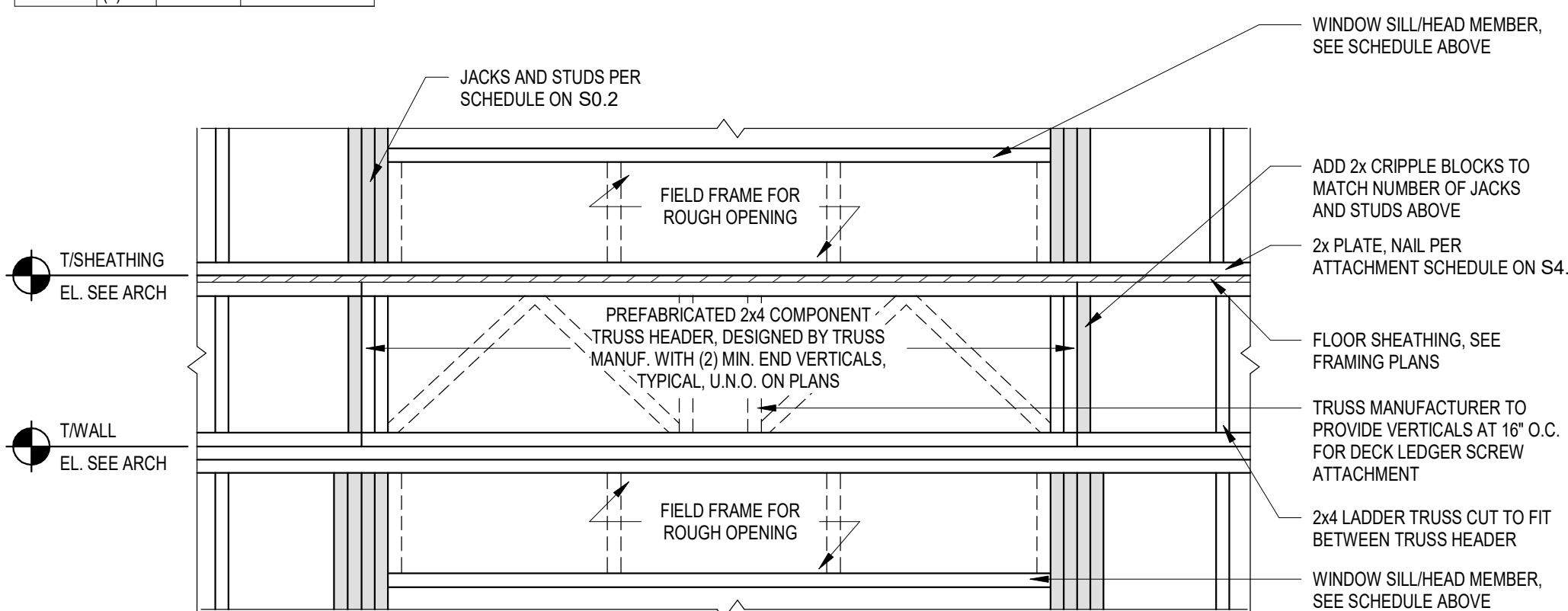
SCALE: N.T.S.

02.01.18

WINDOW SILL/HEAD SCHEDULE			
ROUGH OPENING	SIZE	TOE NAILS EACH END	WIND CRITERIA (ZONE 5 ASD)
56'-4"	(1) 2x6	2	90 MPH
57'-4"	(1) 2x6	3	
58'-4"	(2) 2x6	3	
59'-4"	(2) 2x6	4	

#### NOTES:

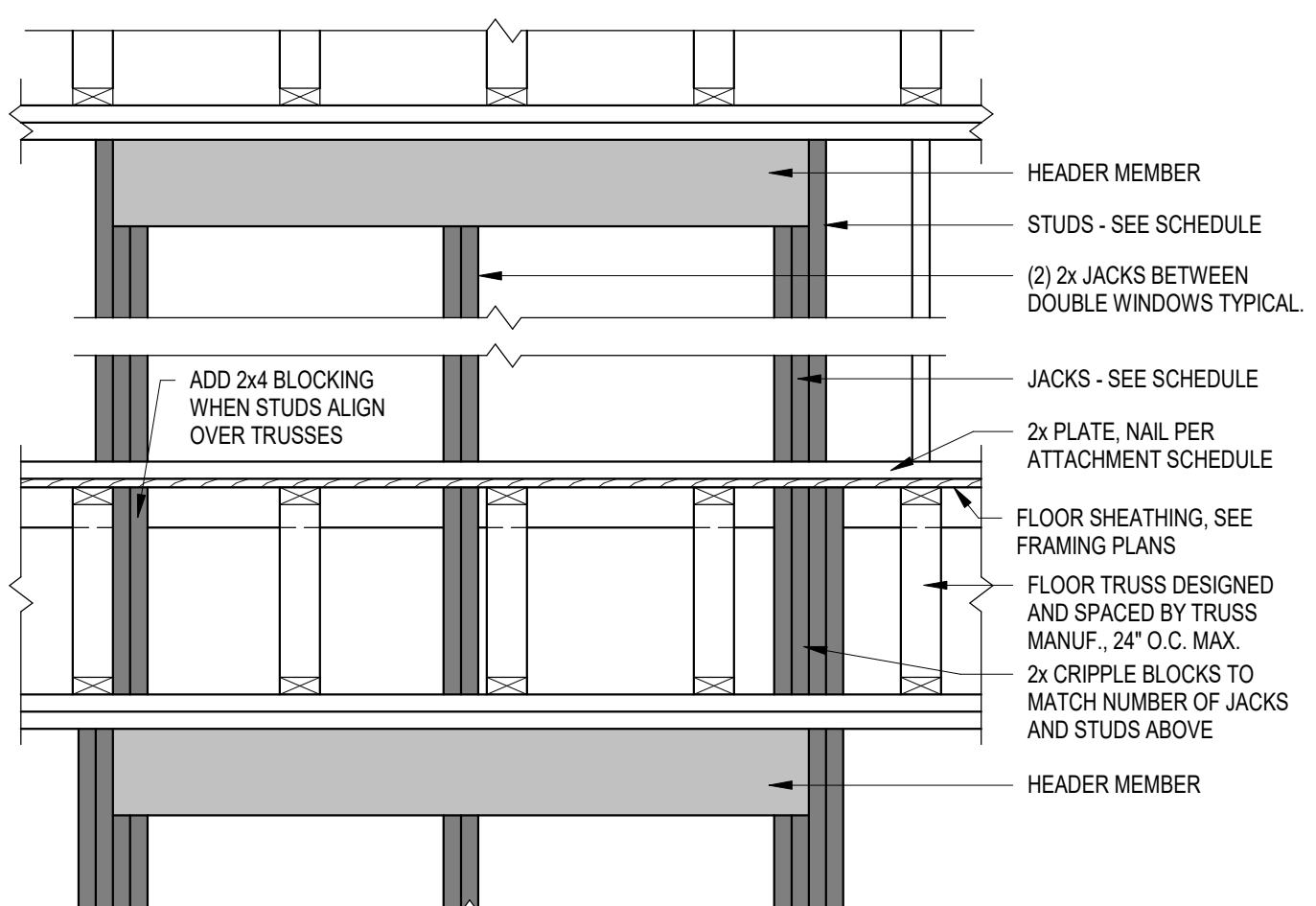
- TRUSS MANUFACTURER TO DESIGN FOR 90 PLF ABOVE OF WALL WEIGHT.
- TRUSS MANUFACTURER TO DESIGN FOR 200 PLF AT DECK LEDGER U.N.O. ON PLAN.



### 2 S5.1 TYPICAL FLUSH TRUSS HEADER ELEVATION

3/4\"/>

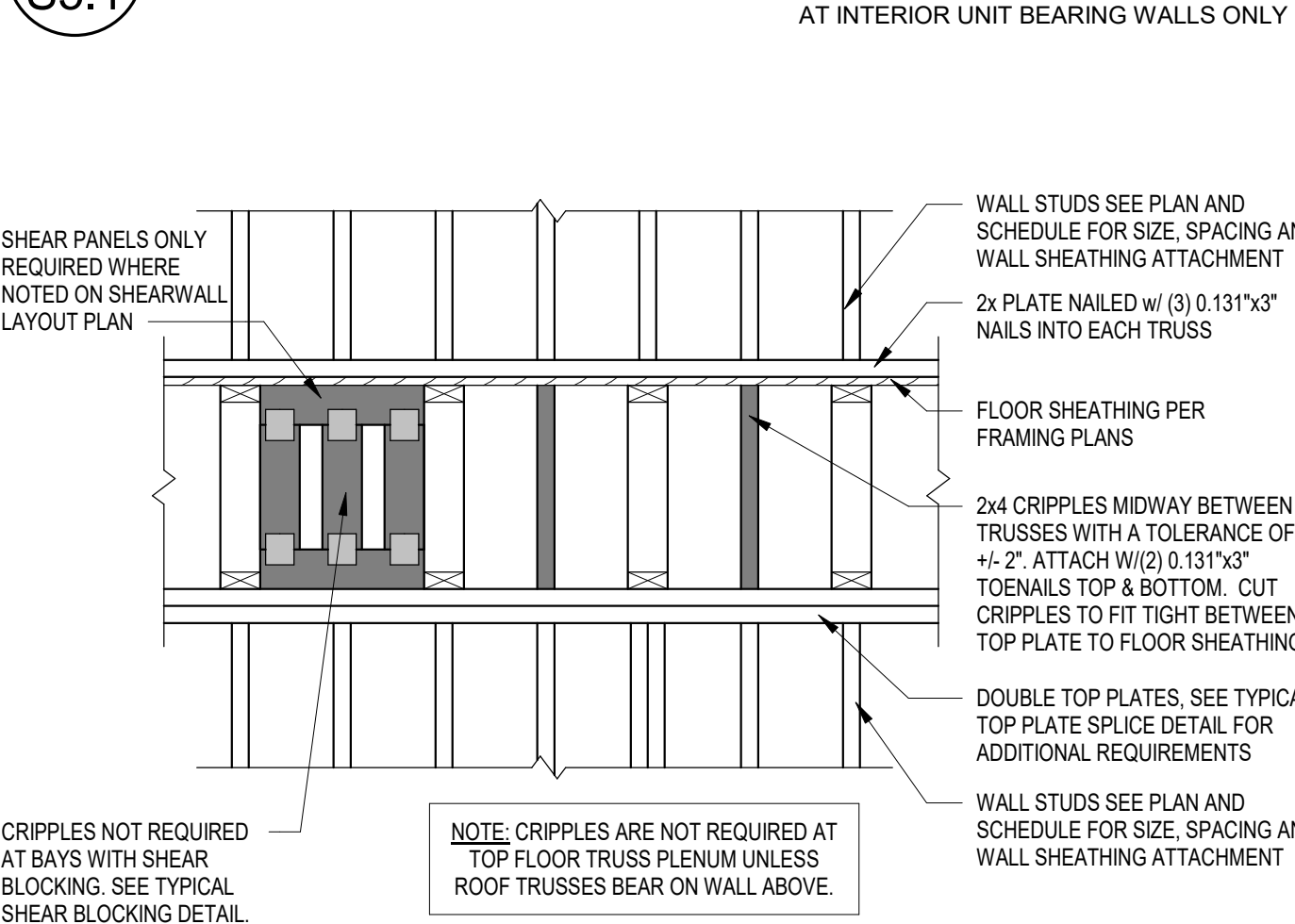
01.23.24  
AT EXTERIOR WALLS



### 3 S5.1 TYPICAL DROPPED HEADER DETAIL

3/4\"/>

01.06.21



### 4 S5.1 INT. UNIT BEARING WALL CRIPPLE DETAIL

SCALE: N.T.S.

10.17.17



Seal

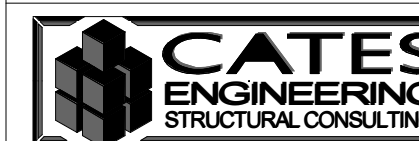


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Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

Drawing Title:  
TYPICAL WOOD FRAMING  
DETAILS

**S5.1**

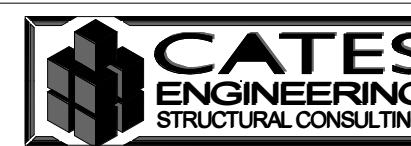
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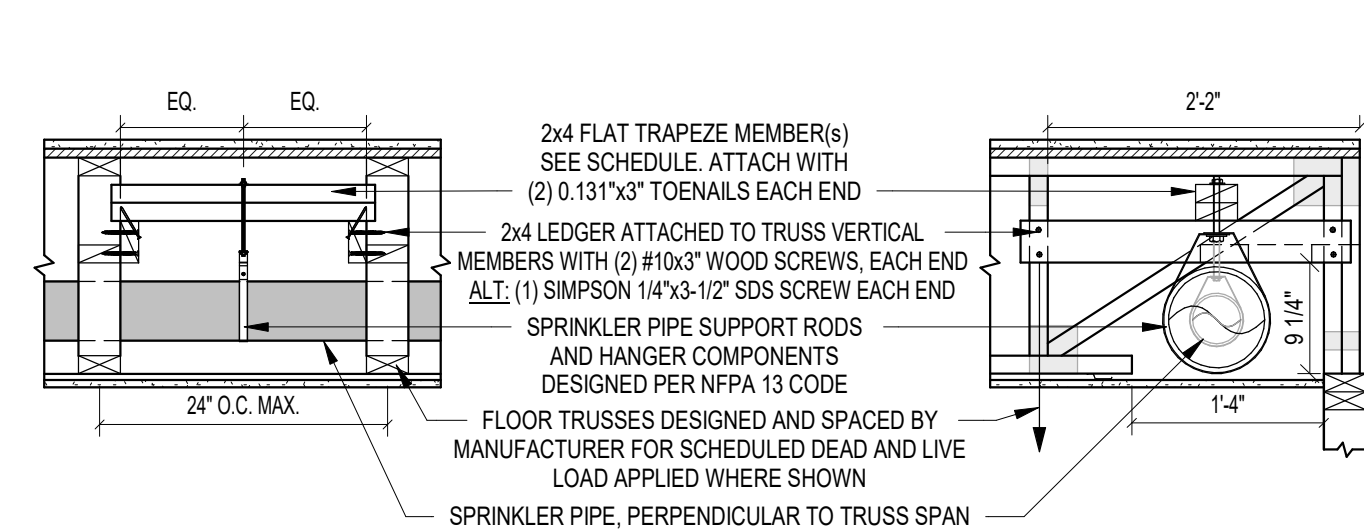
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Daleville Town Center Apartments III  
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in Daleville, Virginia

Drawing Title:  
TYPICAL WOOD FRAMING  
DETAILS

S5.2

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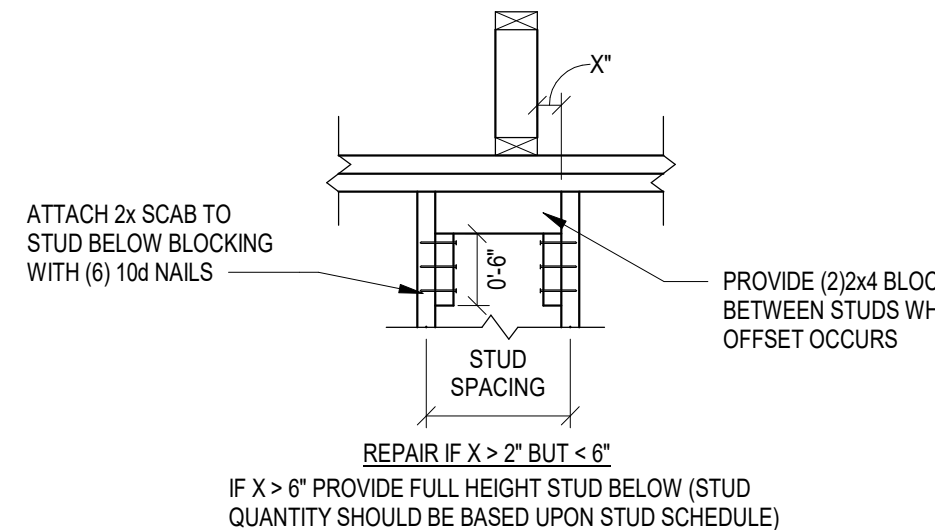
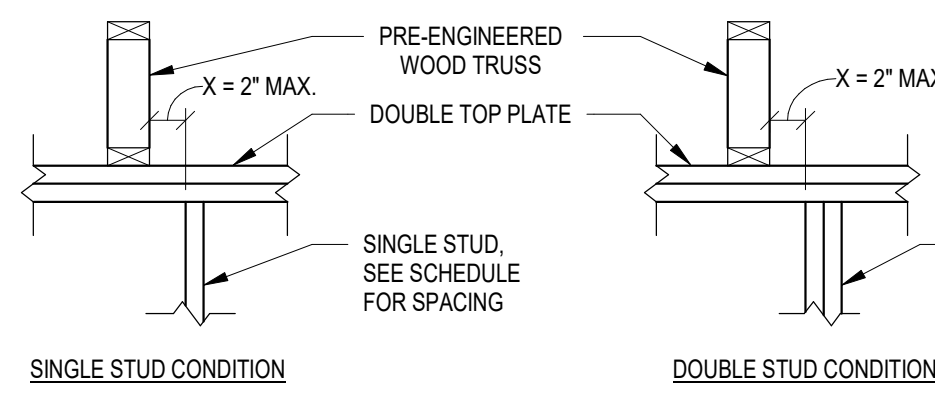


SCHEDULE 40/10 - SPRINKLER PIPE LOADING									
PIPE INSIDE DIA (INCHES)	WET PIPE WT. (PLF)	HANGER SPACING (FEET)	DEAD LOAD REACTION (1)	DEAD LOAD REACTION (2)	TOTAL LOAD REACTION	TRAPEZE MEMBER	POINT LOAD ON TRUSS	POINT LOAD ON TRUSS	POINT LOAD ON TRUSS
			(LBS)	(LBS)					
4.0	16.40	11.78	14.0	230	165	250	480	415	244
5.0	23.47	17.30	14.0	309	242	250	578	492	244
6.0	31.69	23.03	14.0	444	322	250	694	572	244
8.0	47.70	40.08	14.0	668	561	250	918	811	244

- NOTES:
- THE DEAD LOADS FROM THE SPRINKLER HANGERS SHALL BE APPLIED ANYWHERE ALONG THE TOP OR BOTTOM CHORD MEMBER AND ARE IN ADDITION TO ALL OTHER LOADING REQUIREMENTS.
  - IN ADDITION, TRUSSES SHALL ALSO BE DESIGNED FOR A SHORT TERM (24+2.0), 250 POUND LIVE LOAD APPLIED AT THE HANGER LOCATION ANYWHERE ALONG THE TOP OR BOTTOM CHORD MEMBERS, NON-CONCURRENT WITH ANY OTHER LIVE LOADS.
  - TRAPEZE AND LEDGER MEMBERS SHALL BE SPIKE OR BETTER.
  - IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE, NOTIFY AND OBTAIN ANY REQUIRED DESIGN MODIFICATIONS FROM THE TRUSS MANUFACTURER FOR HANGER LOADS OR CONDITIONS WHERE PIPES RUN PARALLEL WITH TRUSSES WHICH WILL REQUIRE MORE THAN ONE HANGER ALONG ITS LENGTH.

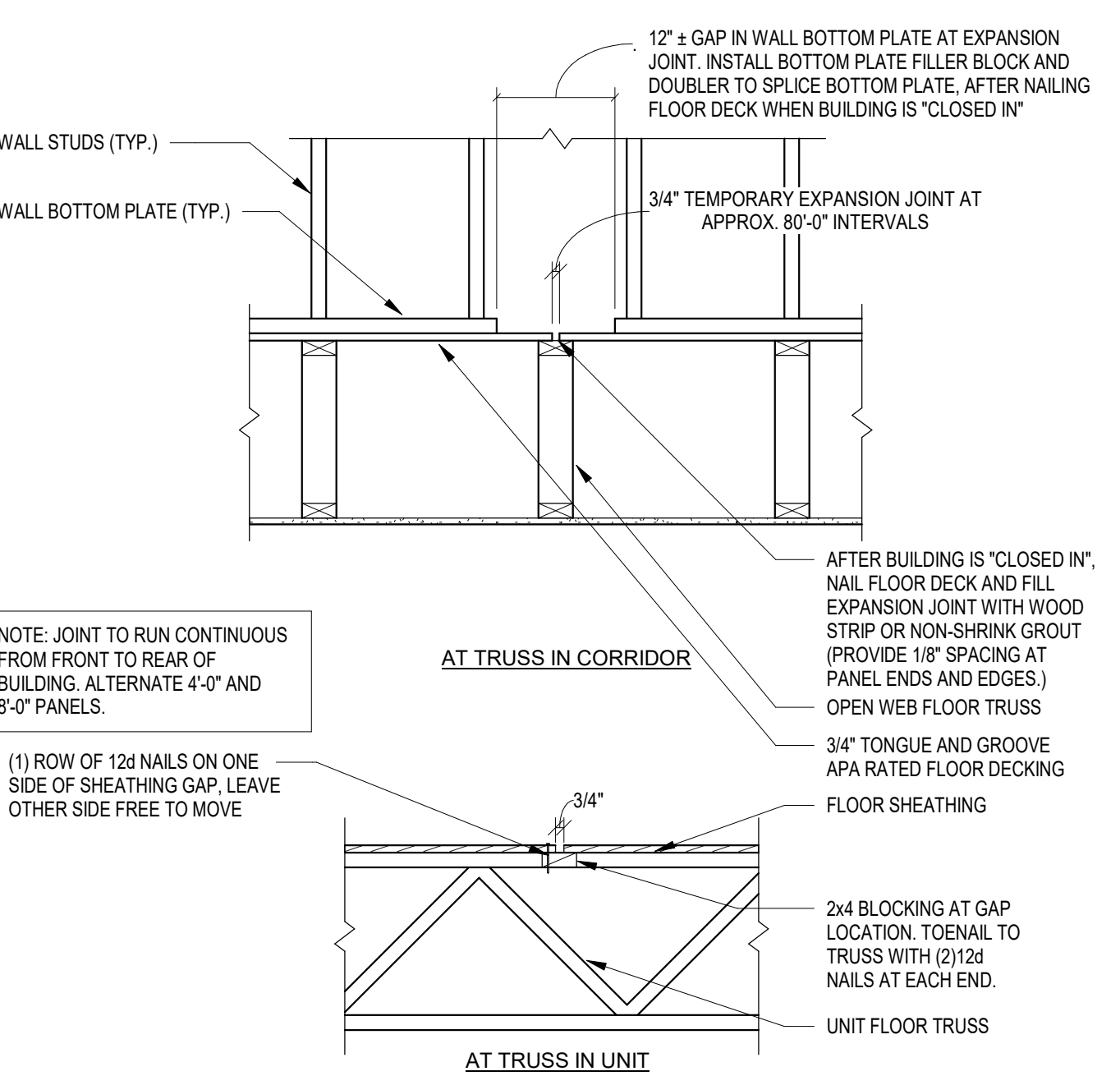
### 1 SPRINKLER PIPE MAIN LINE LOADING

SCALE: N.T.S.



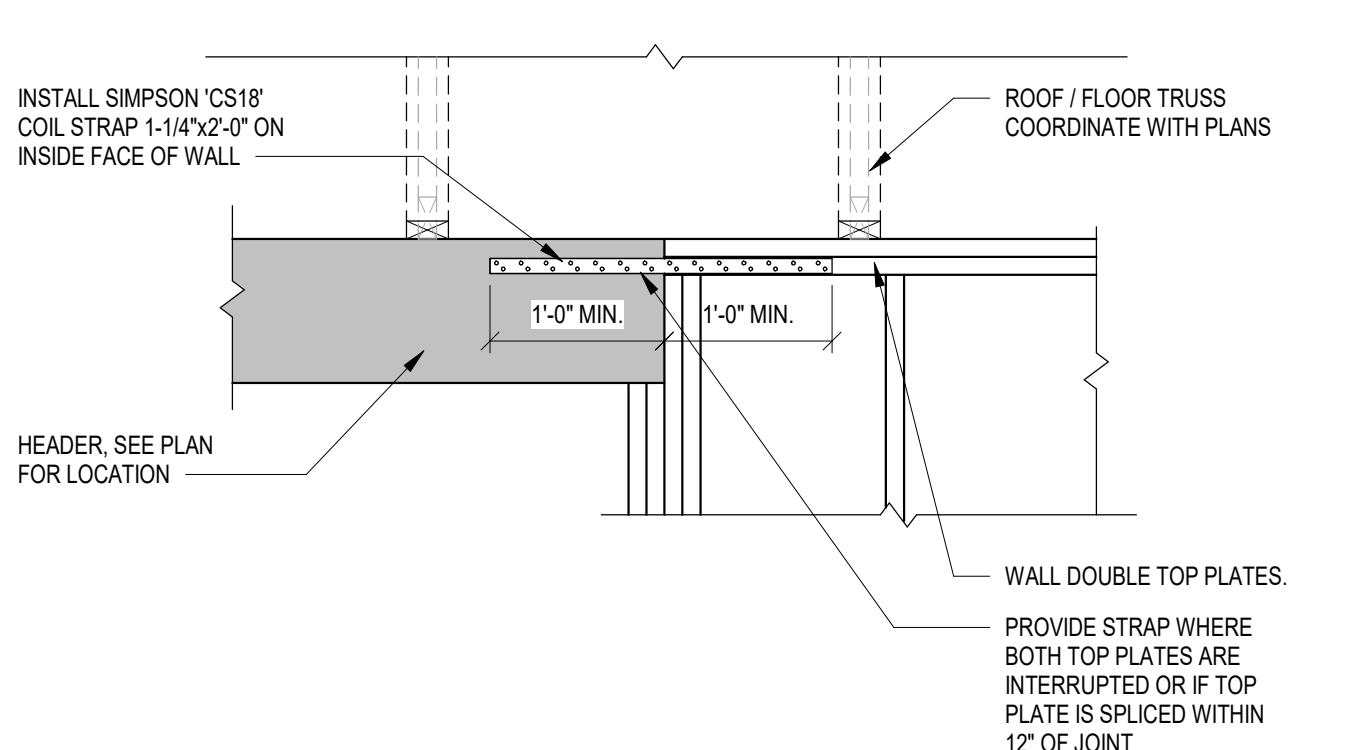
### 2 GUIDE FOR MAXIMUM TRUSS OFFSET

SCALE: N.T.S.



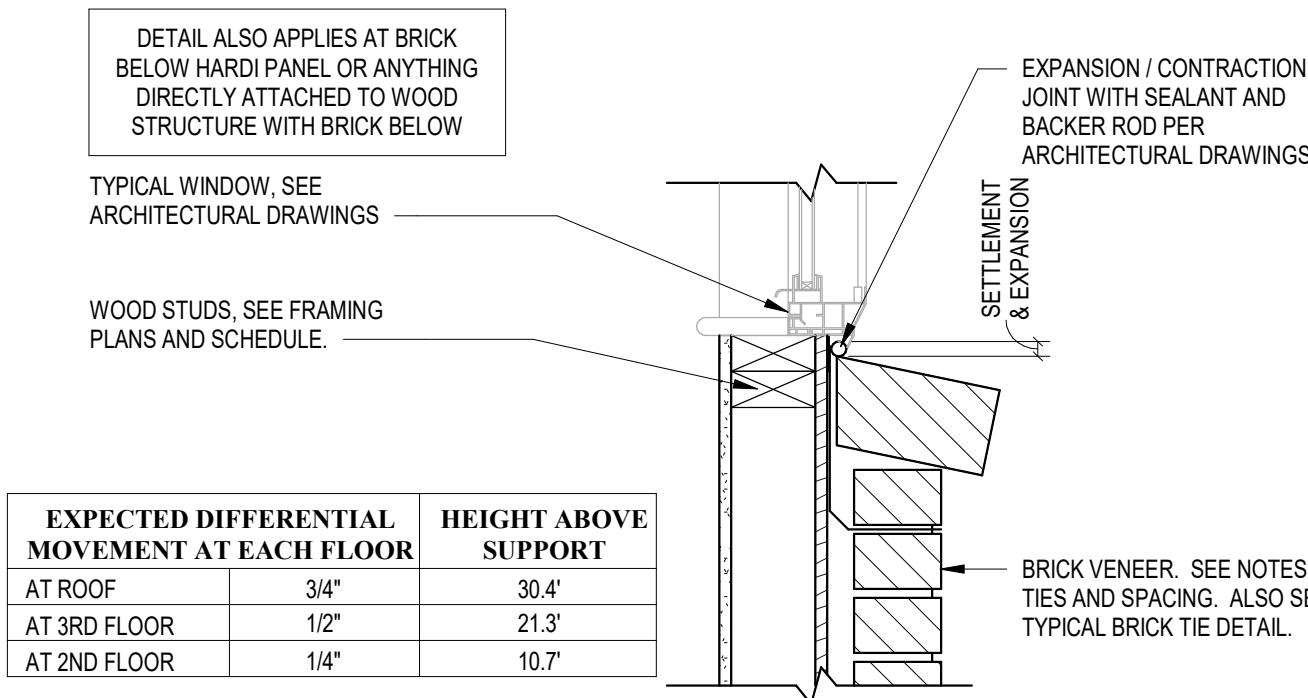
### 3 TEMPORARY FLOOR SHEATHING EXPANSION JOINT

SCALE: N.T.S.



### 4 STRAP WHERE EXT. WALL TOP PLATES INTERRUPTED

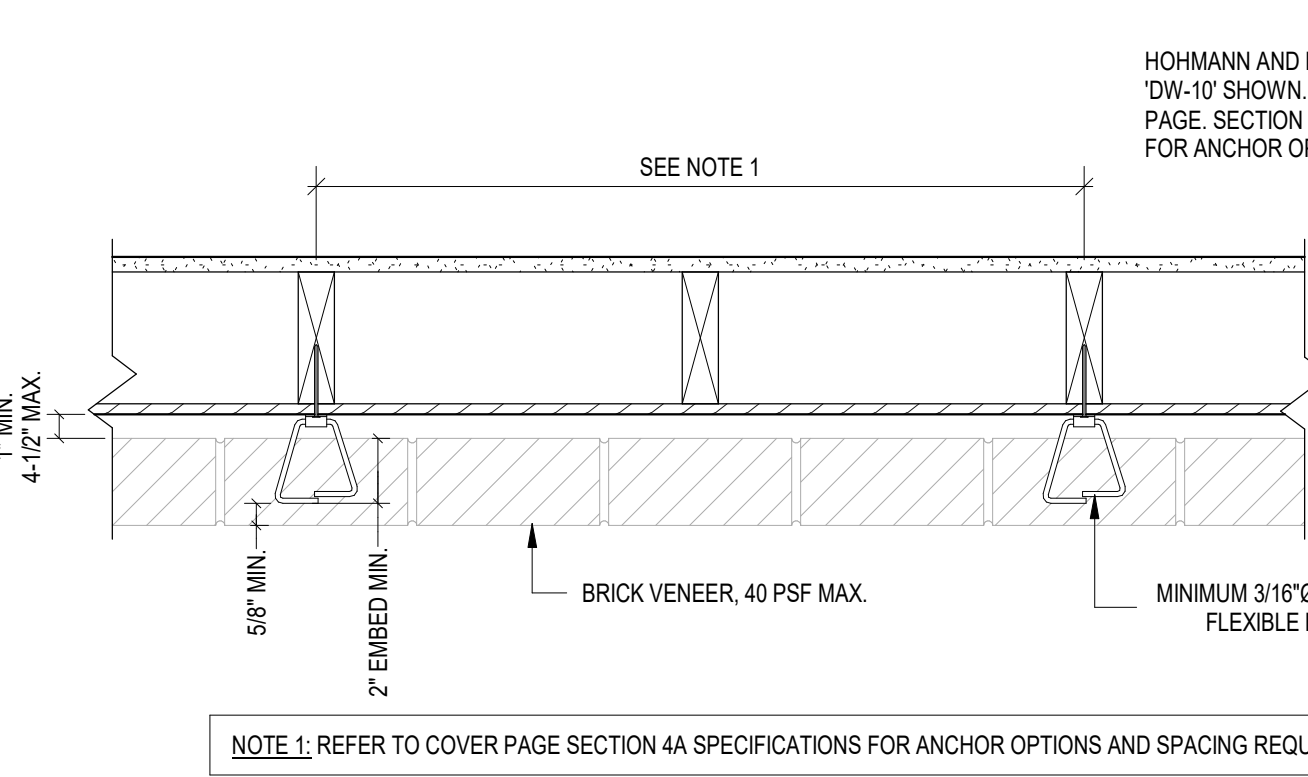
SCALE: N.T.S.



- TYPICAL NOTE TO ARCHITECT/CONTRACTOR:
- FOR BRICK VENEER CONSTRUCTION OVER 25'-0", CARE MUST BE TAKEN TO ALLOW FOR EXPANSION OF BRICK AND SETTLEMENT OF WOOD FRAMING. THE DIFFERENTIAL SETTLEMENT BETWEEN BRICK AND WOOD FRAMING VARIES PER FLOOR IN A WOOD FRAMED STRUCTURE. SEE REQUIRED GAP SCHEDULE. WHERE WINDOWS OVERLAP BRICK VENEER, AN ADEQUATE EXPANSION JOINT MUST BE PROVIDED.
  - WE RECOMMEND THAT THE JOINTS SHOULD NOT BE CAULKED UNTIL THE ENTIRE BUILDING IS LOADED WITH THE DEAD LOAD FROM THE DRYWALL AND GYPSUM.
  - ARCHITECT SHALL PROVIDE NECESSARY DETAILS TO ACCOMMODATE THE ANTICIPATED DIFFERENTIAL MOVEMENTS. THESE DETAILS MAY REQUIRE LARGER GAPS BETWEEN BRICK AND ANY ELEMENTS ATTACHED TO WOOD STRUCTURE TO ACCOUNT FOR CAULK COMPRESSIBILITY.

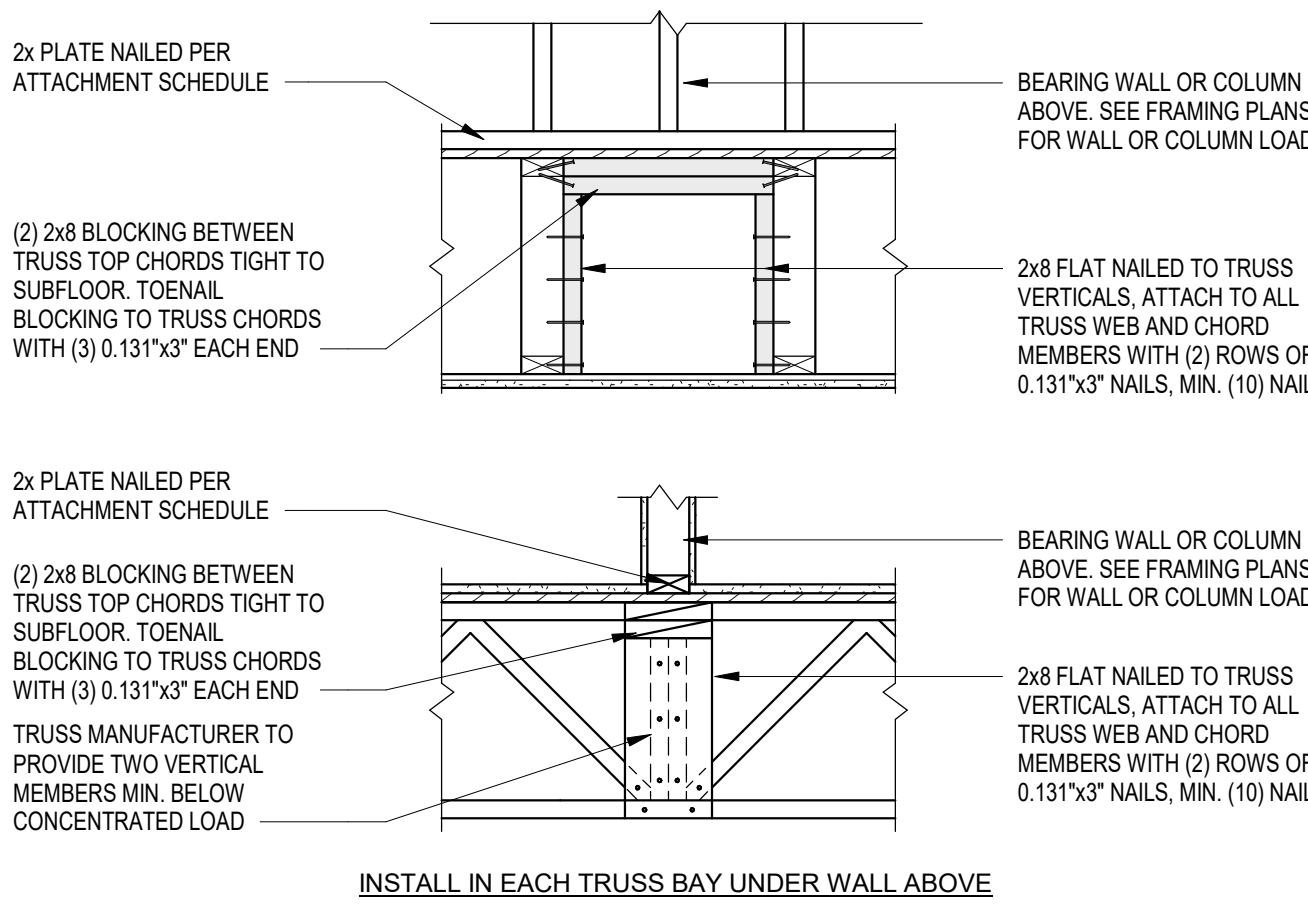
### 5 EXPECTED SETTLEMENT AND EXPANSION JOINT

SCALE: N.T.S.



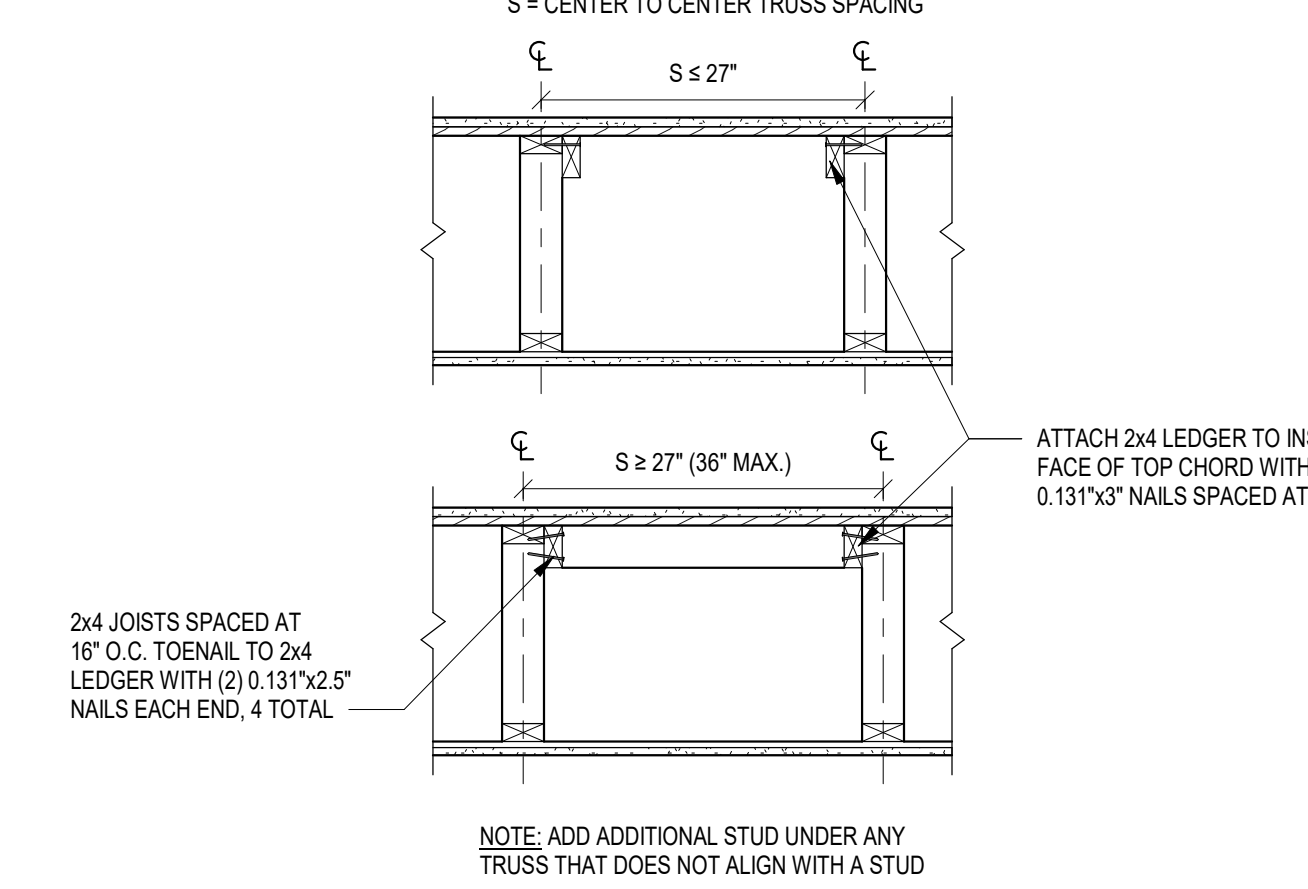
### 6 ADJUSTABLE BRICK ANCHOR - WOOD STUD BACKUP

SCALE: N.T.S.



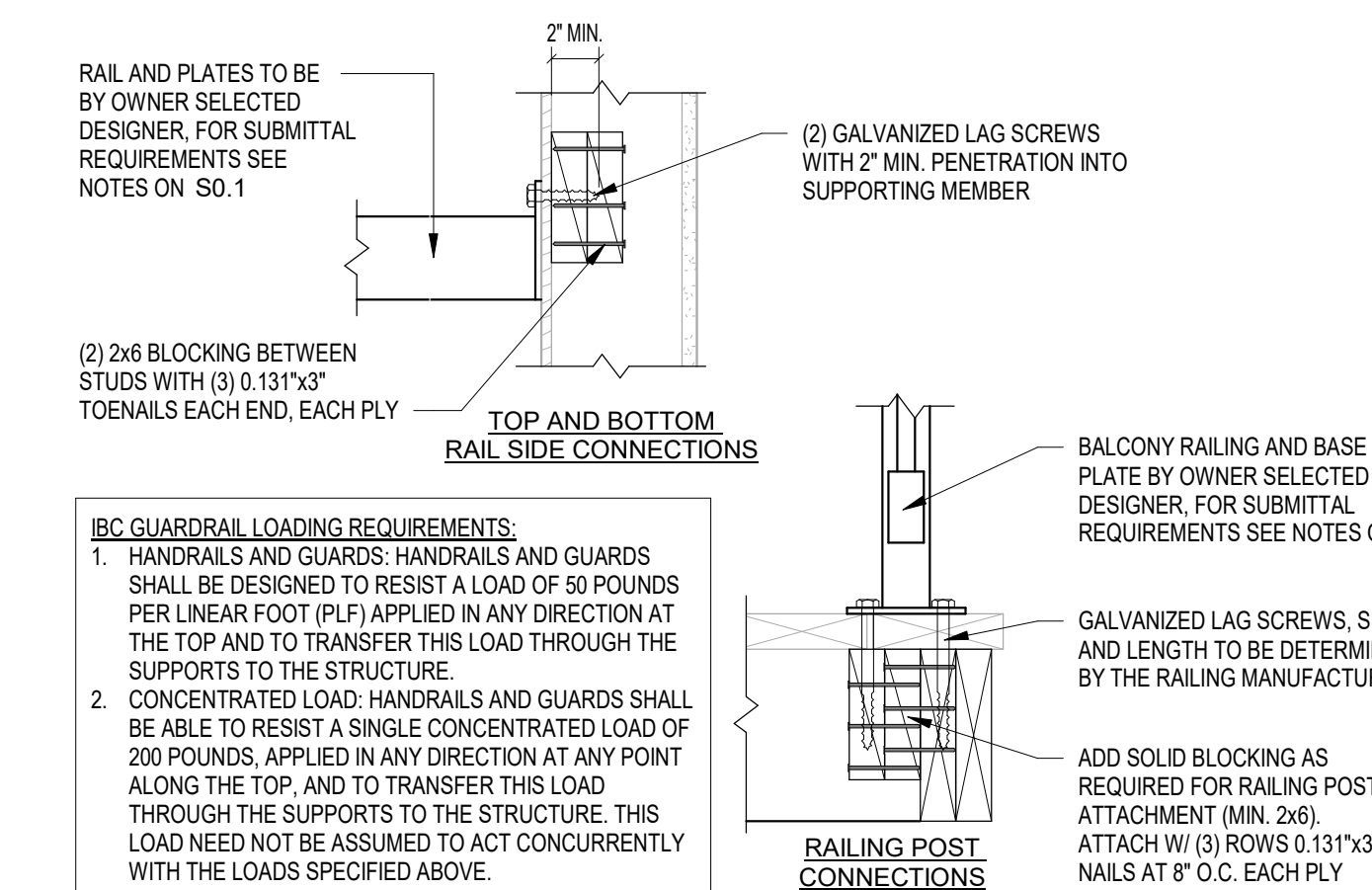
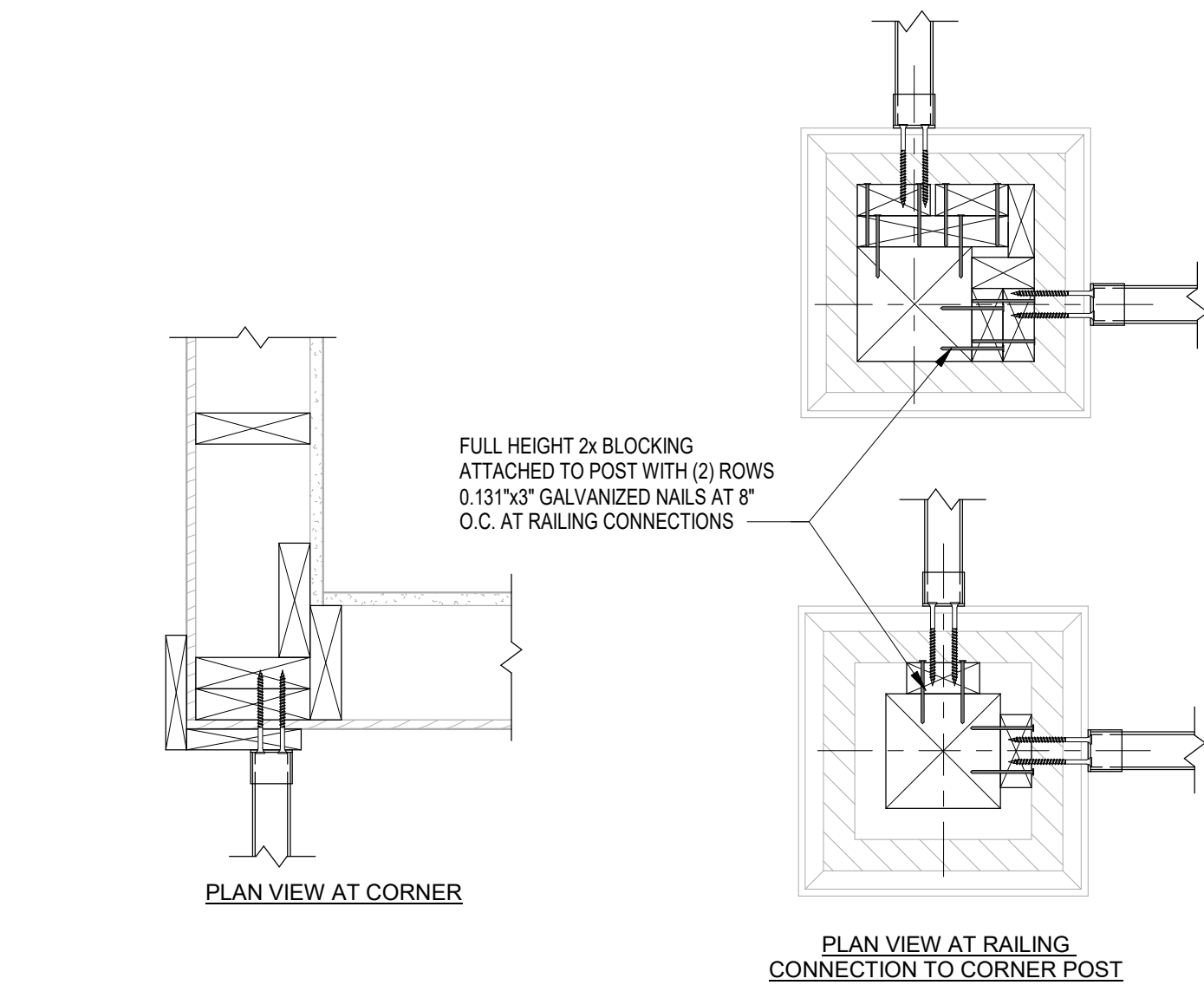
### 7 TRUSS SUPPORTING BEARING WALL ABOVE

SCALE: N.T.S.



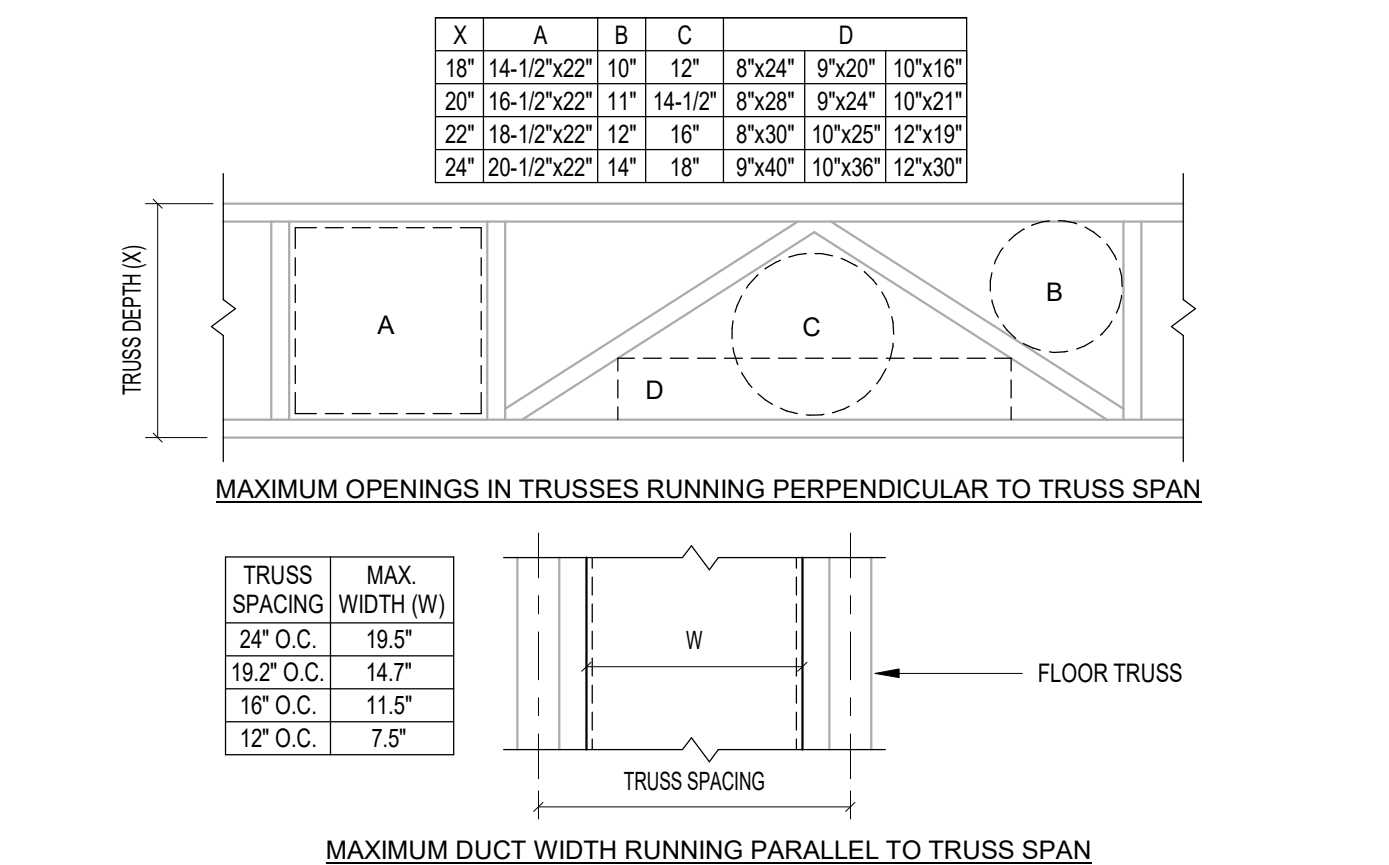
### 8 TRUSSES SPACED AT GREATER THAN 24" O.C.

SCALE: N.T.S.



### 9 BALCONY GUARDRAIL CONNECTION DETAIL

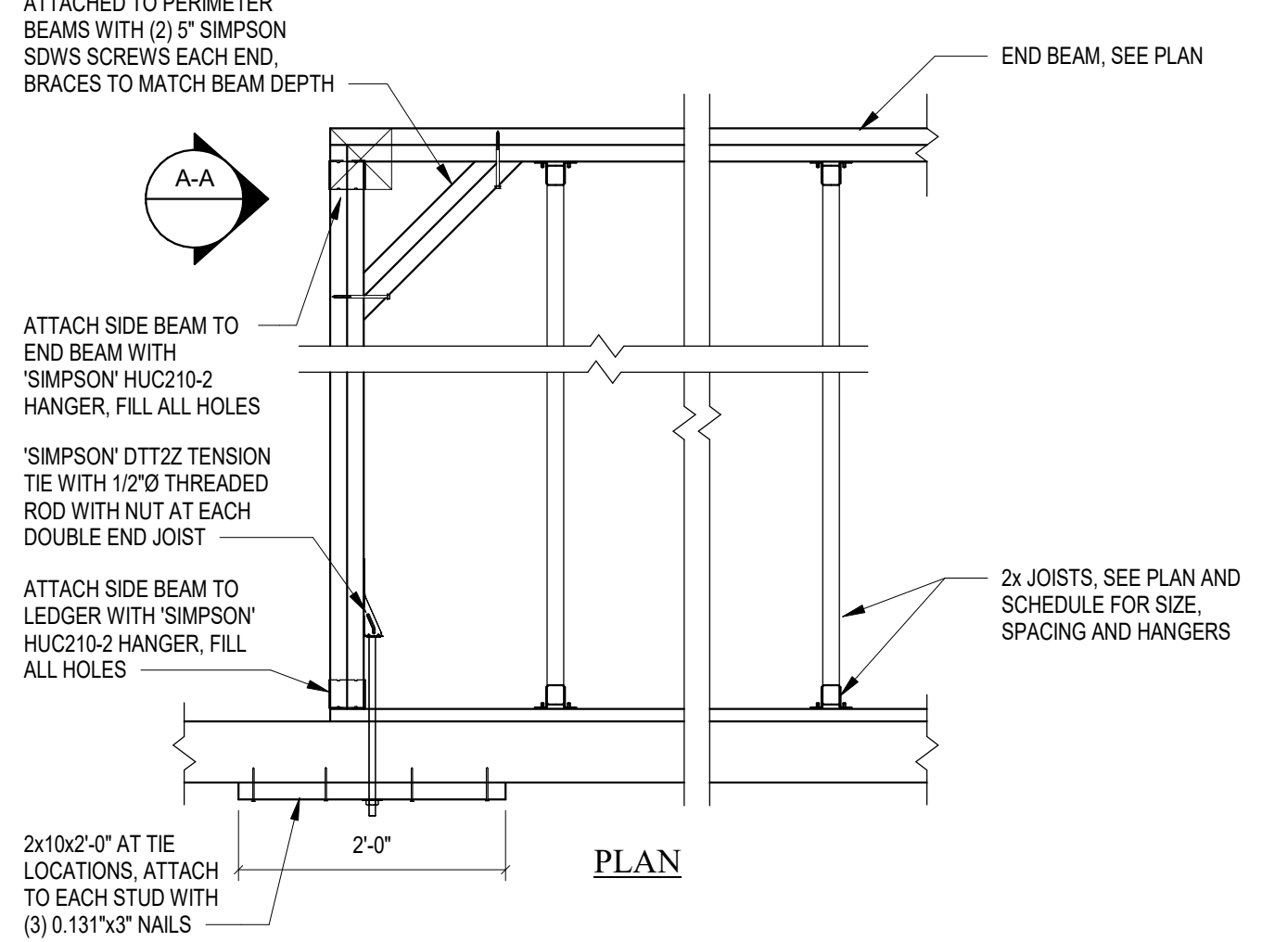
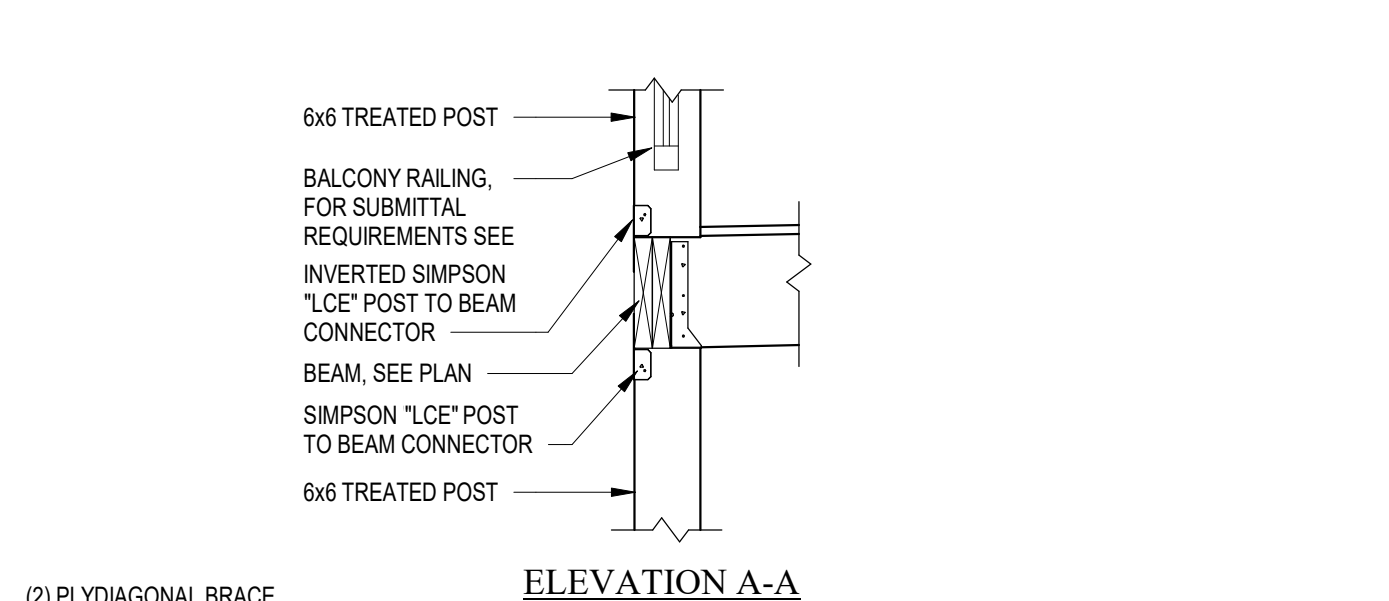
SCALE: N.T.S.



- NOTES:
- DUCT SIZES SHOWN ARE BASED ON TYPICAL MAXIMUM PANEL SIZES AND COULD VARY FROM ACTUAL TRUSS DESIGN. DUCT SIZES AND LOCATIONS WILL NEED TO BE COORDINATED WITH TRUSS MANUFACTURER DURING SHOP DRAWING REVIEW.

### 10 TRUSS OPENING GUIDELINES

SCALE: N.T.S.



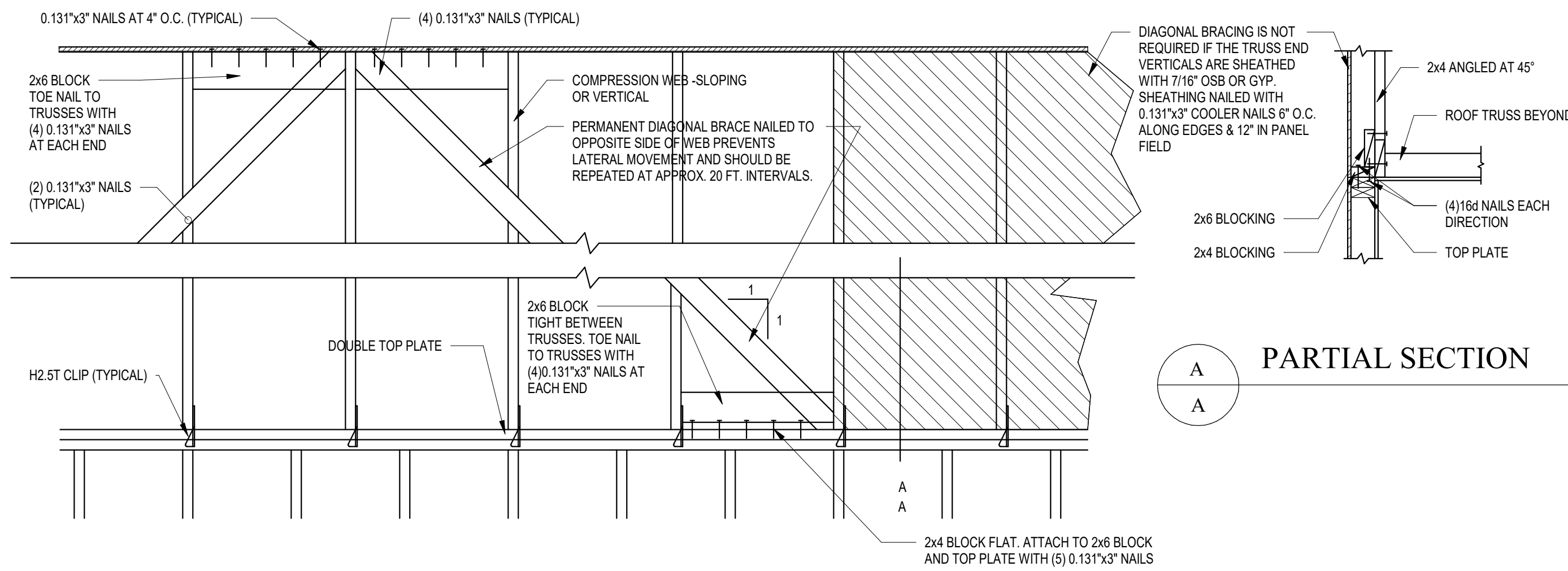
### 11 TYPICAL EXTERIOR BALCONY - PLAN VIEW

SCALE: N.T.S.

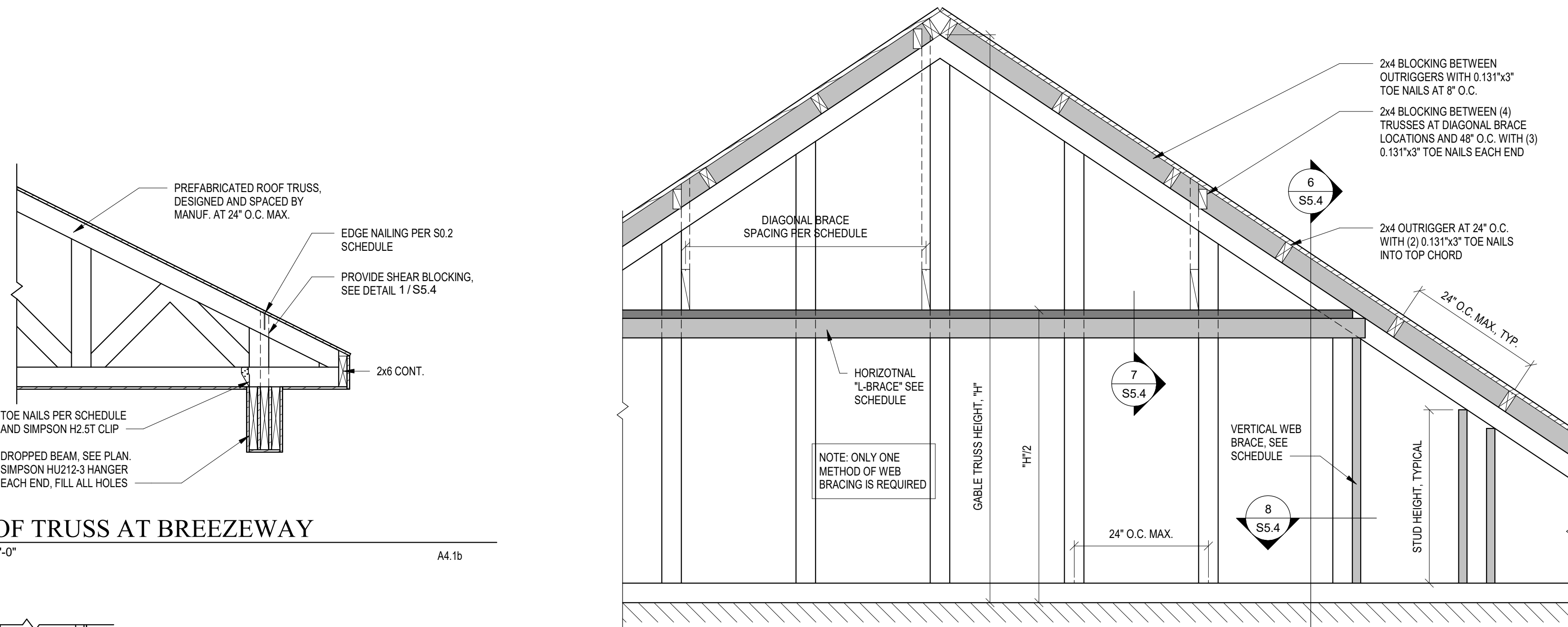




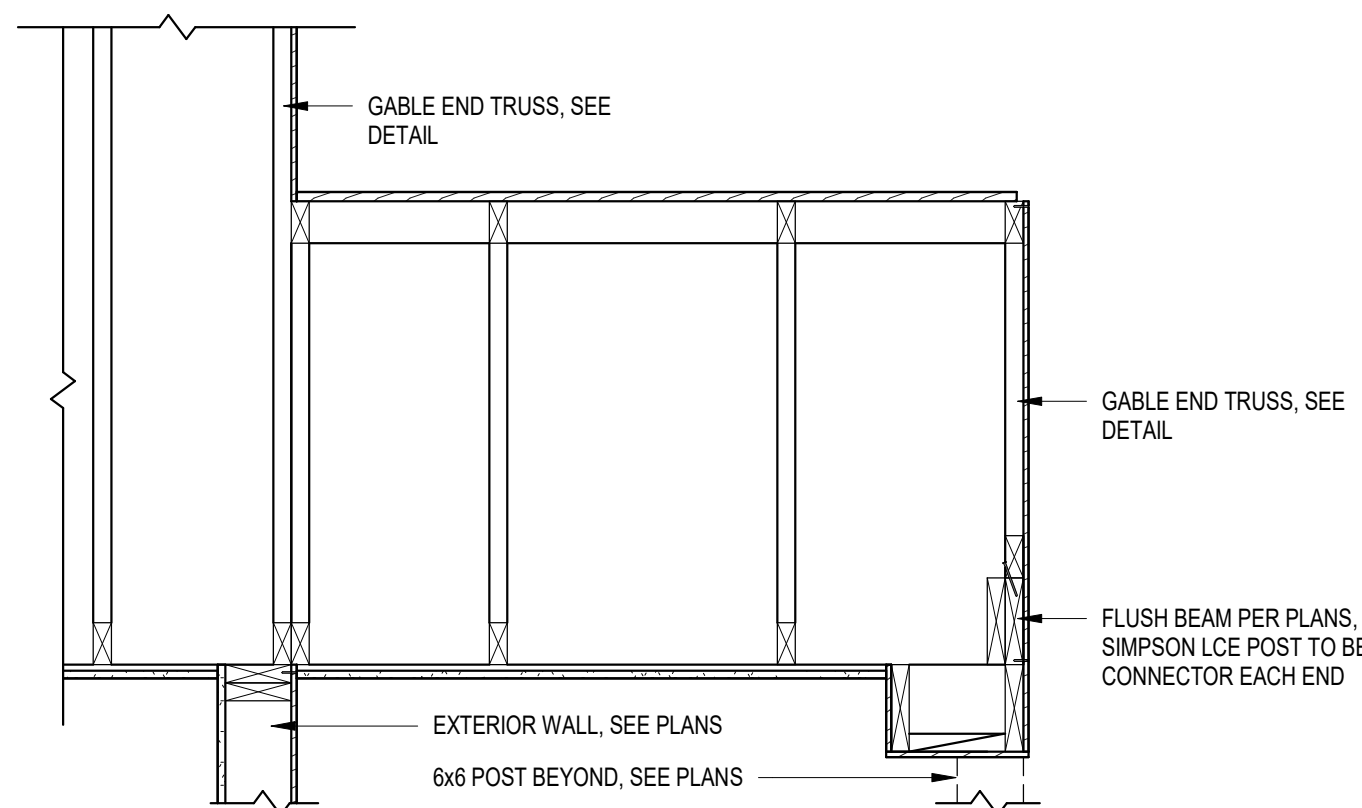




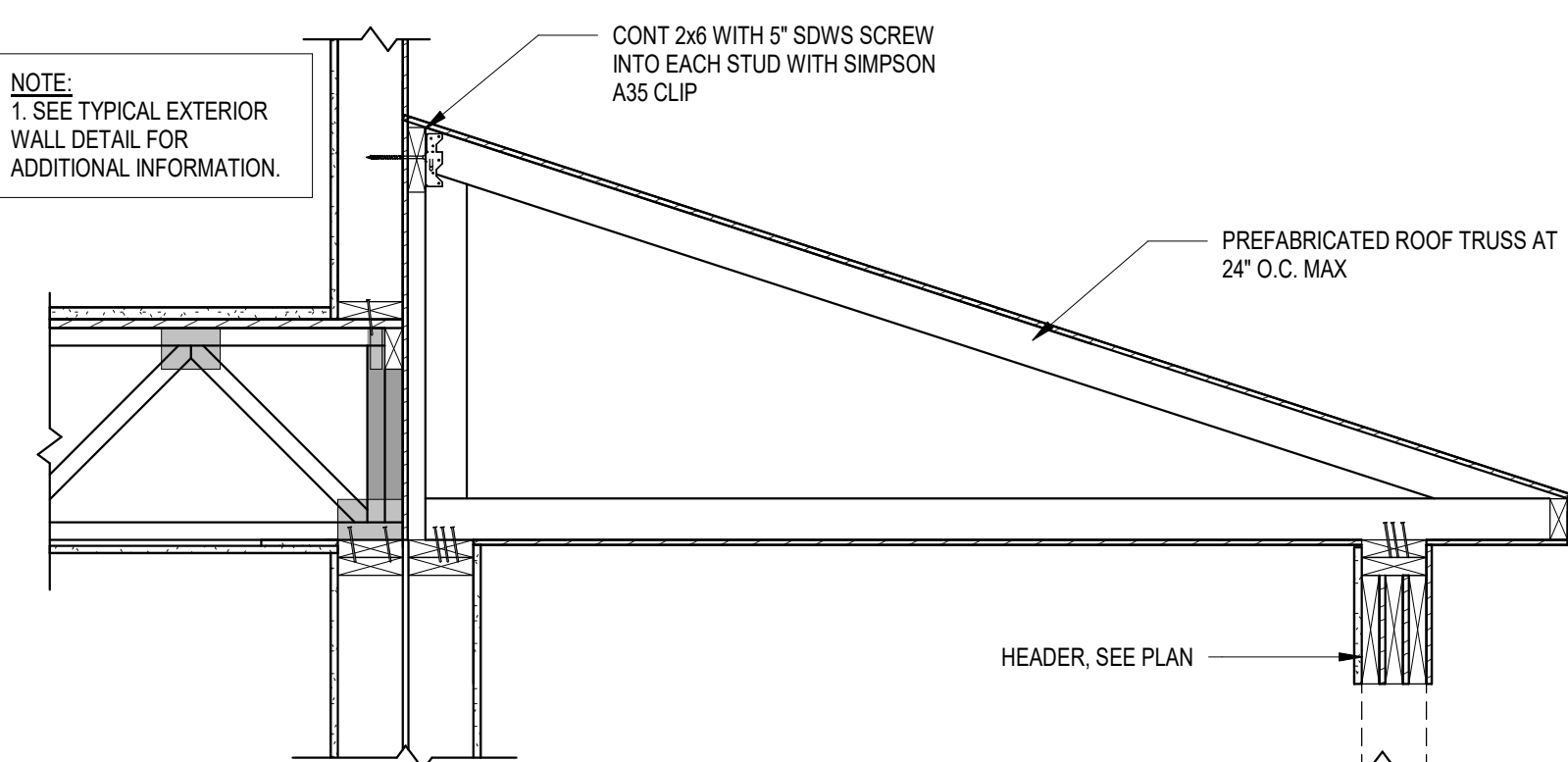
**9**  
**S5.4** TYPICAL ROOF TRUSS BRACING AT CORRIDOR/BREEZEWAY  
 SCALE: N.T.S.



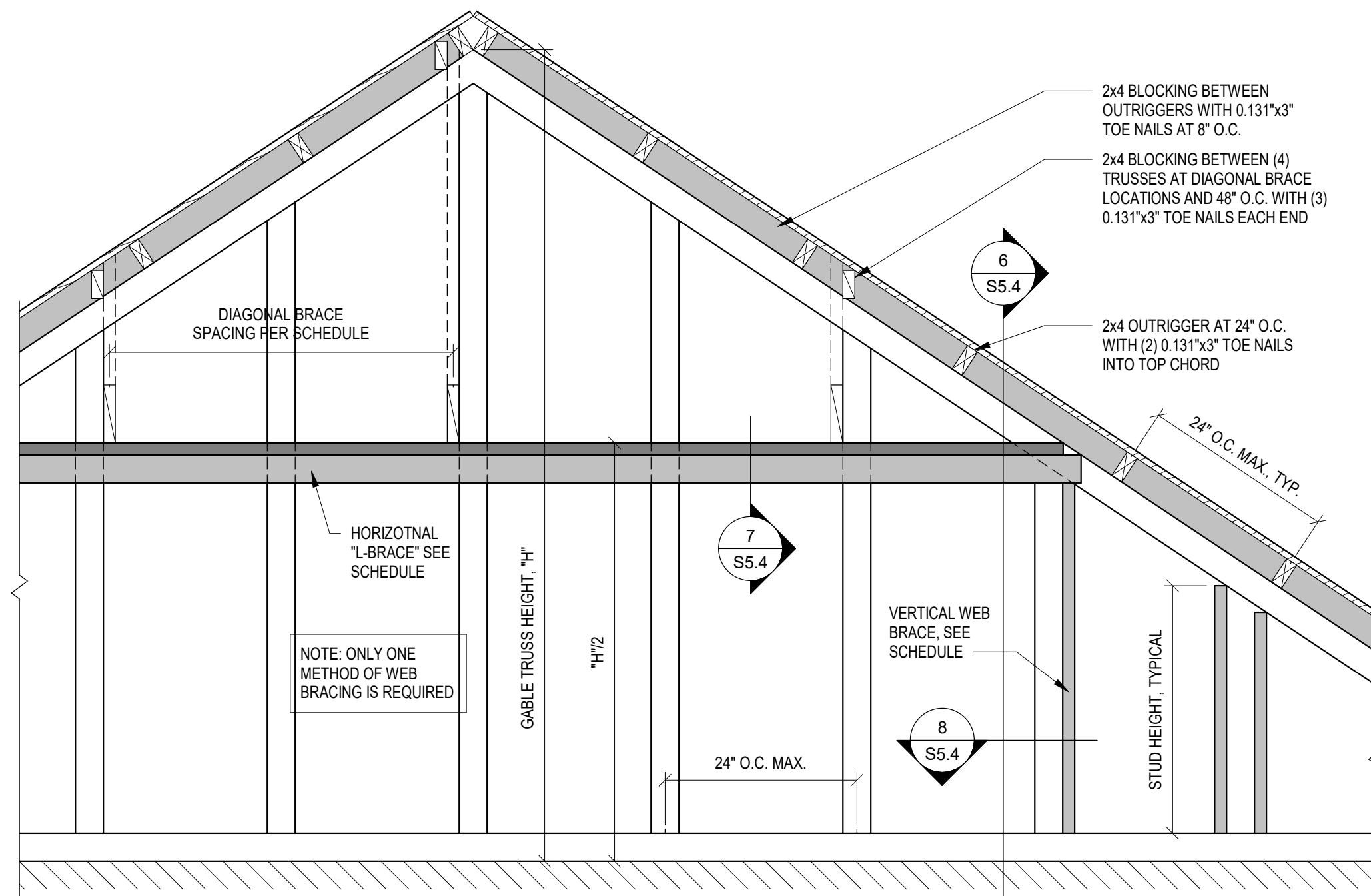
**13**  
**S5.4** ROOF TRUSS AT BREEZEWAY  
 3/4\"/>



**14**  
**S5.4** SECTION AT BUMPOUT ROOF BALCONY  
 3/4\"/>



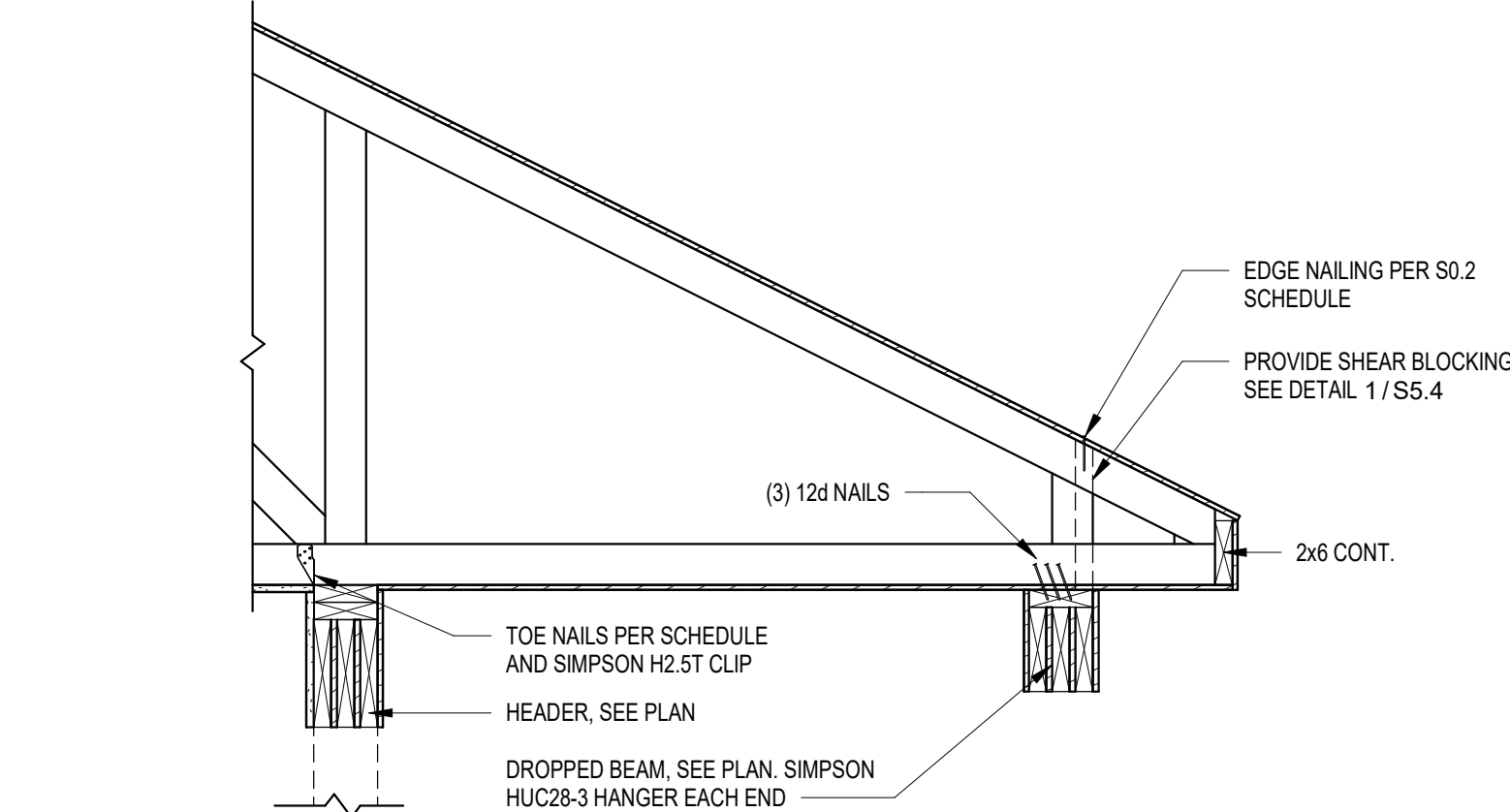
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**S5.4** ROOF TRUSS AT UTILITY ROOM  
 3/4\"/>



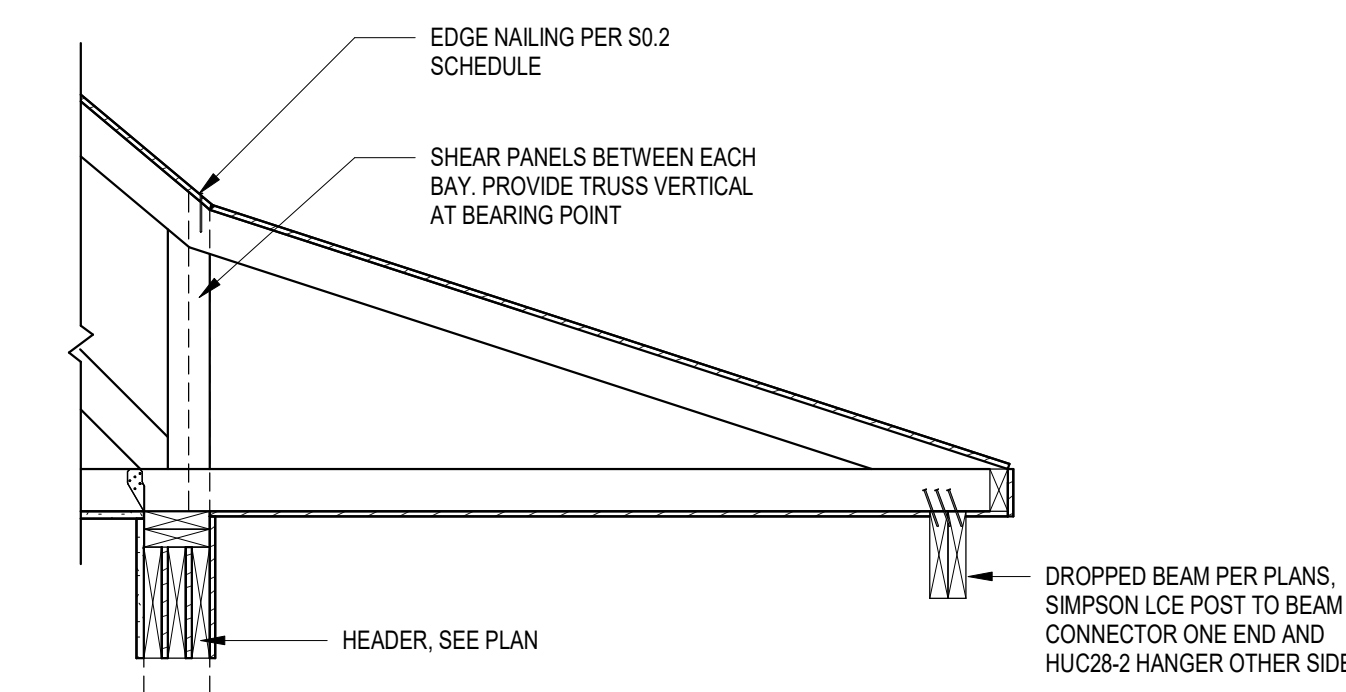
**10**  
**S5.4** GABLE END TRUSS ELEVATION  
 SCALE: N.T.S.

GABLE END TRUSS BRACING OPTIONS <sup>(3)</sup>									
VERTICAL WEB L-BRACING <sup>(1)</sup>					HORIZONTAL L-BRACING <sup>(1)</sup>				
STUD SIZE	SPACING	WITHOUT BRACE	VERTICAL L-BRACE	VERTICAL L-BRACE	WITHOUT DIAGONAL	EVERY 6'-0"	EVERY 4'-0"	DIAGONAL BRACE SPACING <sup>(3)</sup>	
2x4	24\" O.C.	4'-0"	7'-0"	8'-8"	MAX. GABLE HEIGHT, \"H	6'-0"	8'-8"	12'-6"	
2x6	24\" O.C.	4'-0"	10'-6"	12'-6"					

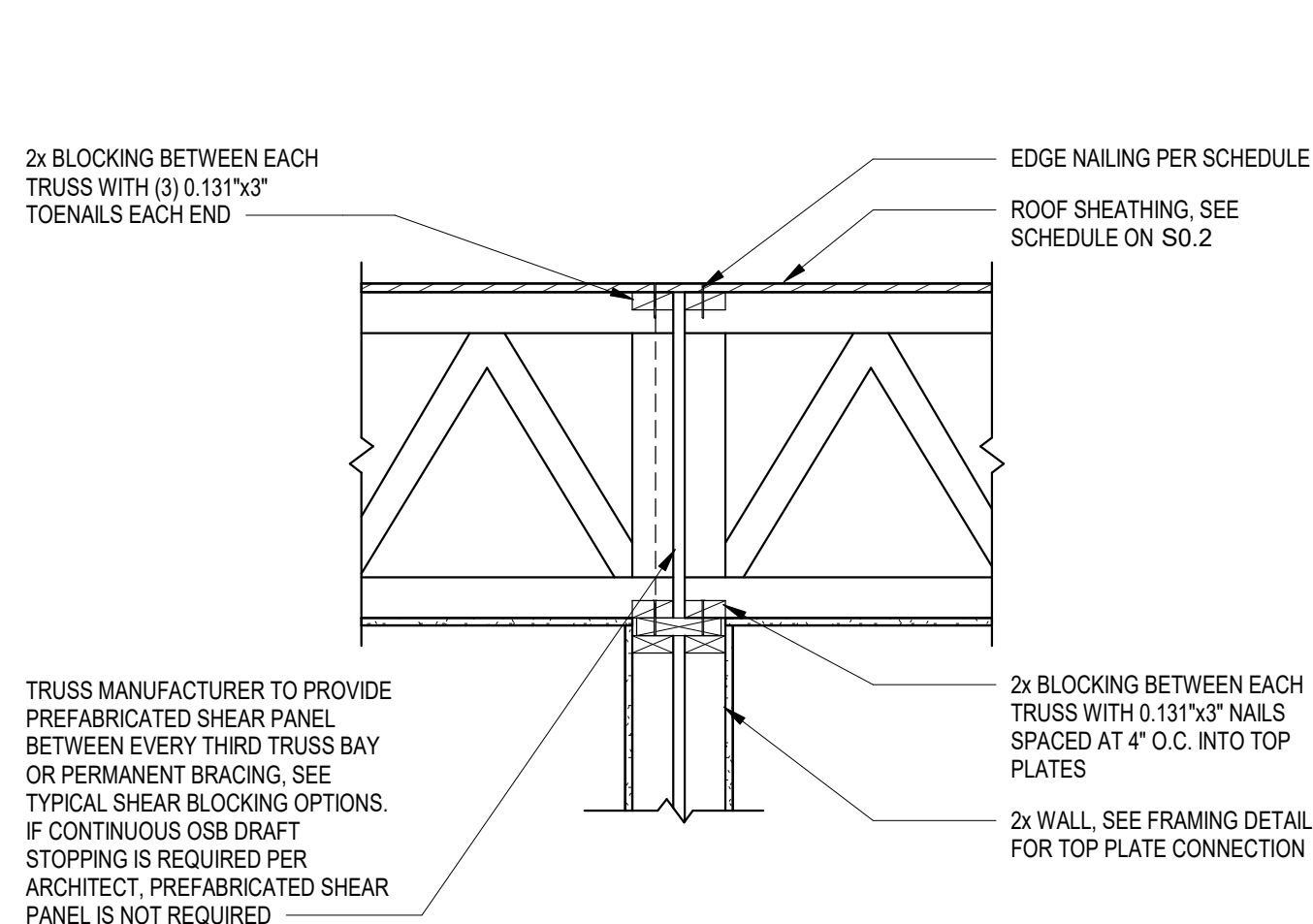
NOTES:  
 1. ONLY ONE METHOD OF BRACING IS REQUIRED, U.N.O.  
 2. ALL BRACING LUMBER SHALL BE SP#2 OR BETTER.  
 3. DESIGN IS BASED ON THE FOLLOWING DESIGN VALUES:  
 MAXIMUM WIND SPEED = 90 MPH  
 MEAN ROOF HEIGHT = 50 FEET  
 CATEGORY I BUILDING, \"H\" = 1.0  
 EXPOSURE CATEGORY = B  
 DURATION LOAD INCREASE = 1.60  
 4. STUD DESIGN IS BASED ON COMPONENTS AND CLADDING LOADS AND BRACING CONNECTIONS ARE BASED ON MWFRS LOADS.  
 5. DIAGONAL BRACES LONGER THAN 6'-0\" REQUIRE A 2x4 T-BRACE ATTACHED TO ONE EDGE WITH 12d NAILS SPACED AT 8\" O.C. AND 3\" MAX. FROM ENDS. T-BRACE MUST COVER 90% OF DIAGONAL BRACE.



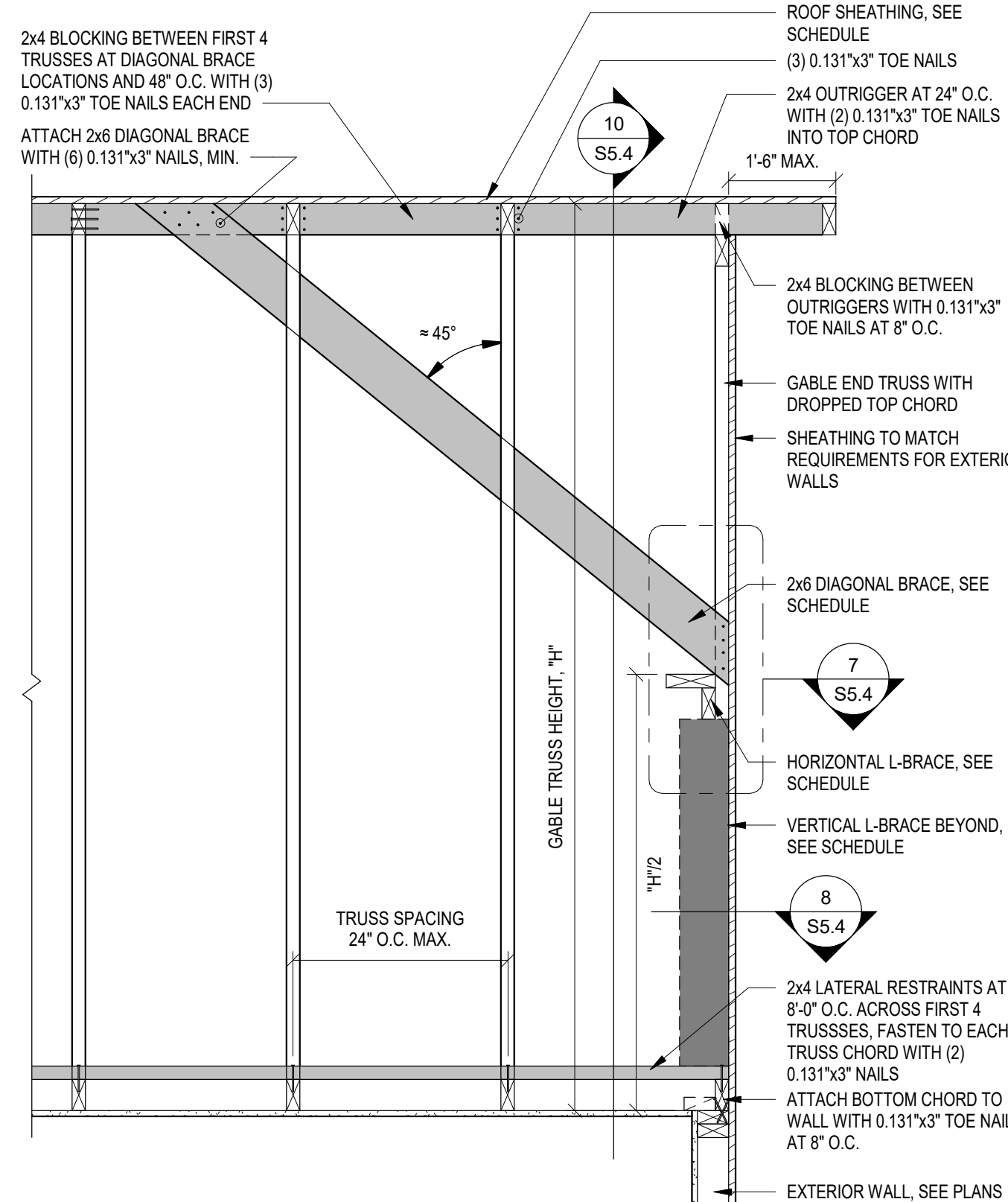
**11**  
**S5.4** ROOF TRUSS AT INSET BALCONY  
 3/4\"/>



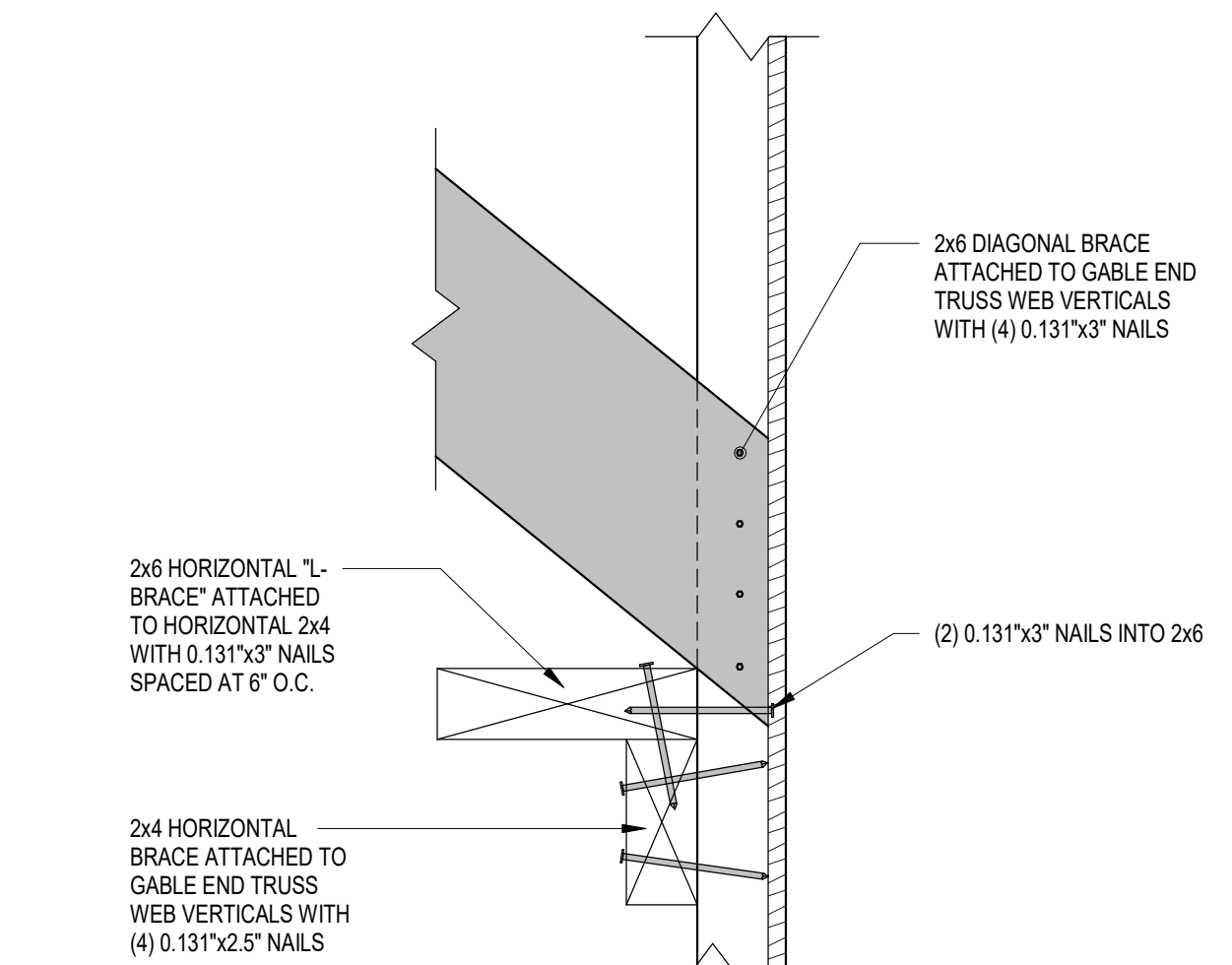
**12**  
**S5.4** ROOF TRUSS AT BALCONY  
 3/4\"/>



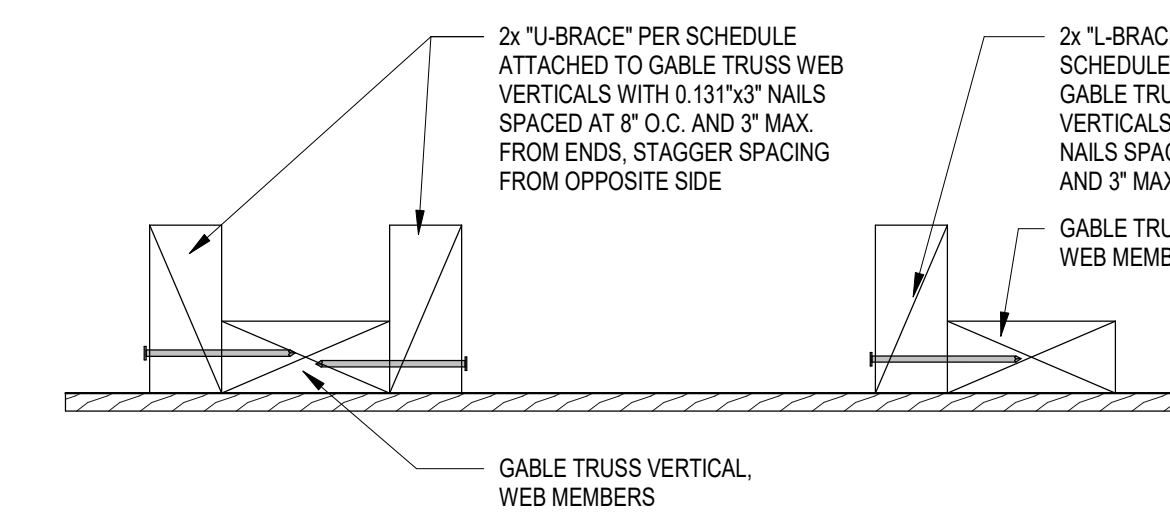
**5**  
**S5.4** ROOF TRUSS AT UNIT SEPARATION WALL  
 SCALE: N.T.S.



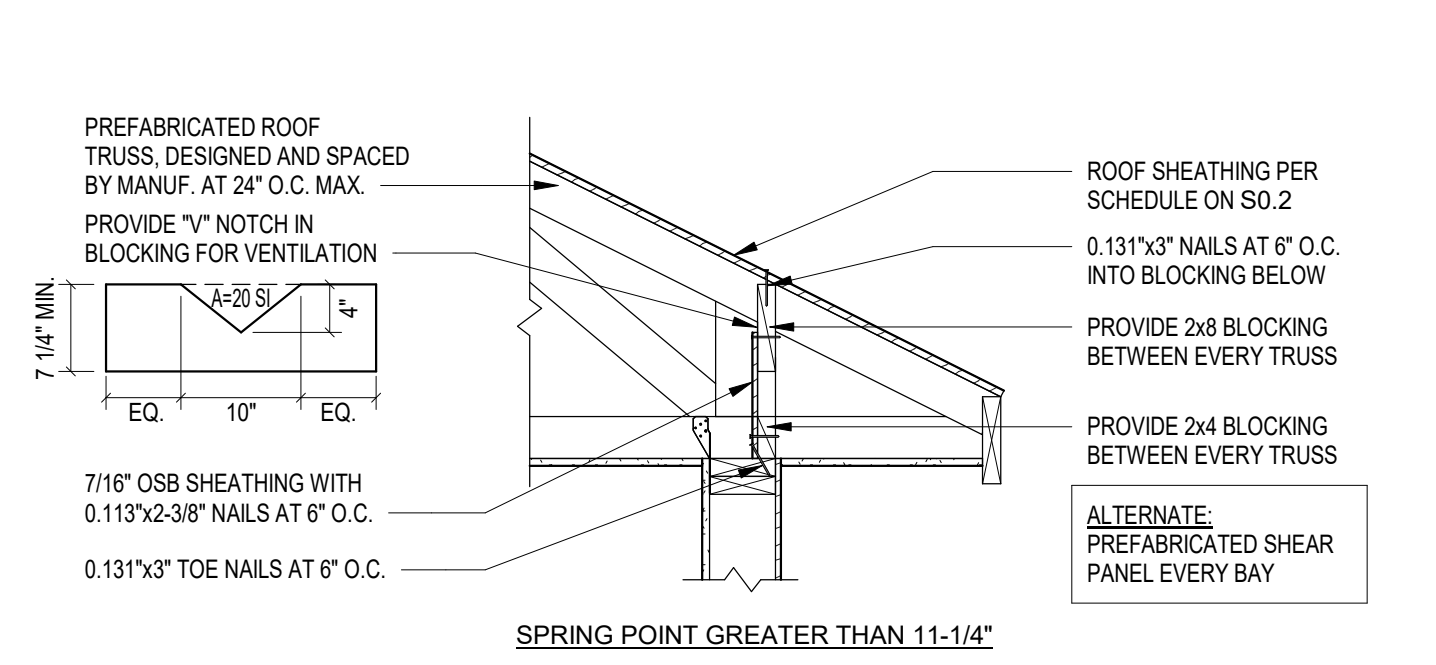
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**S5.4** GABLE END TRUSS BRACING CONNECTIONS  
 SCALE: N.T.S.



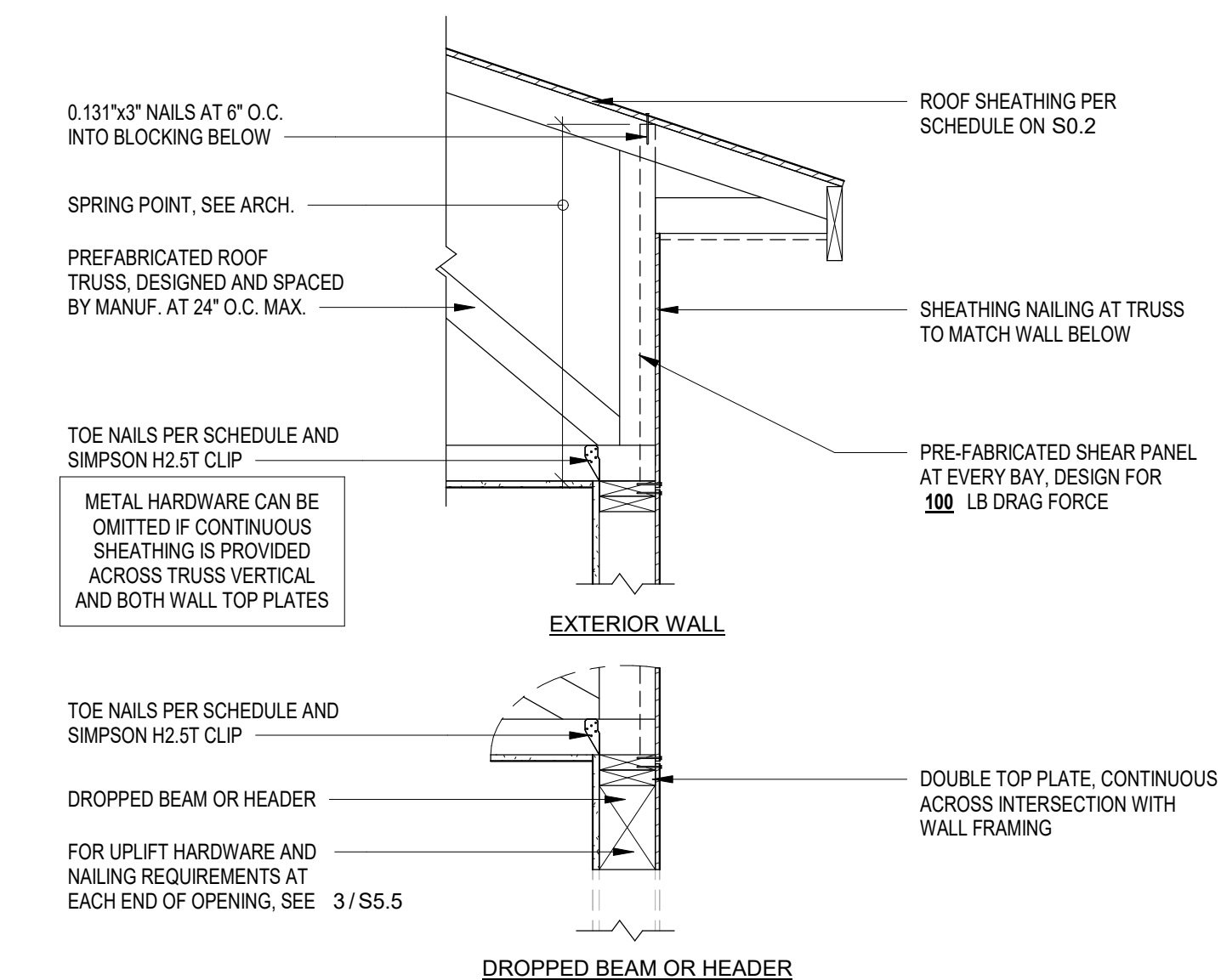
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**S5.4** HORIZONTAL WEB BRACING CONNECTION  
 SCALE: N.T.S.



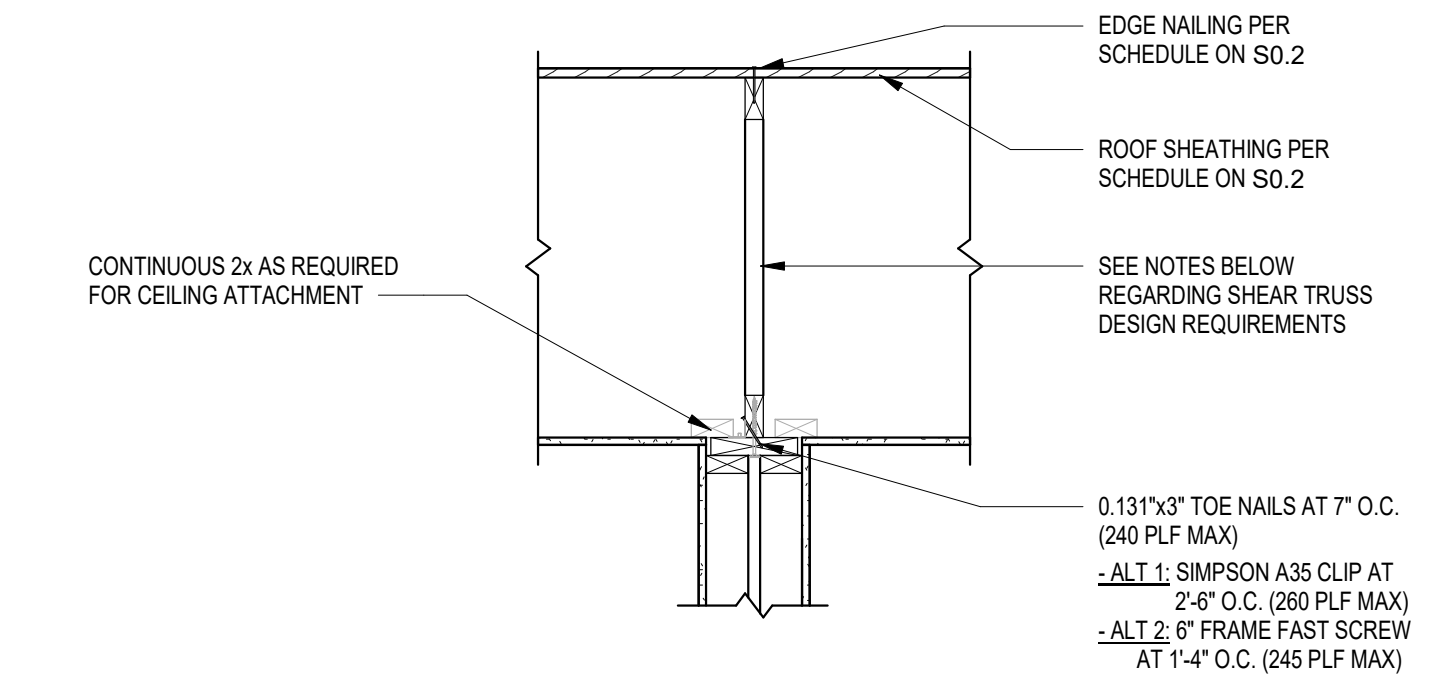
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**S5.4** VERTICAL WEB BRACING CONNECTION  
 SCALE: N.T.S.



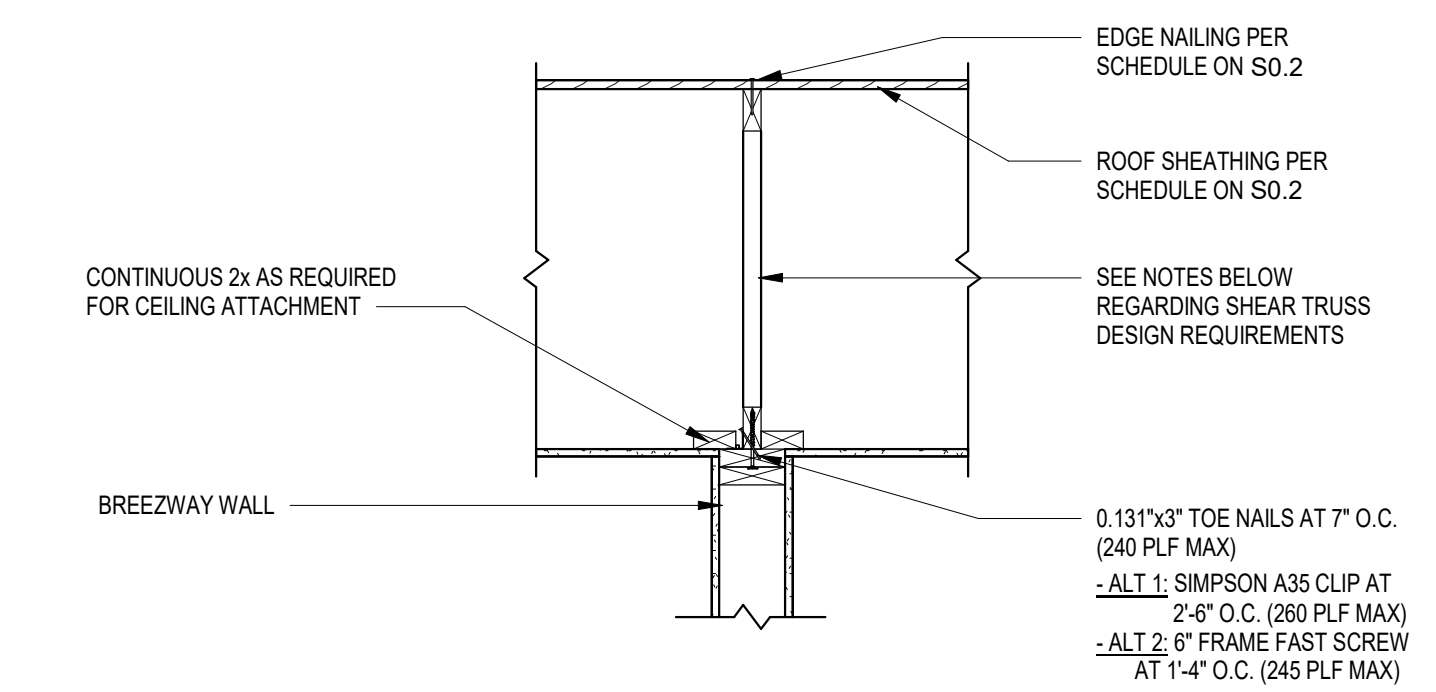
**1**  
**S5.4** TYPICAL PITCHED ROOF TRUSS BEARING  
 3/4\"/>



**2**  
**S5.4** RAISED HEEL ROOF TRUSS WITH SOFFIT  
 3/4\"/>



**3**  
**S5.4** SHEAR TRUSS AT UNIT SEPARATION WALL  
 SCALE: N.T.S.



**4**  
**S5.4** SHEAR TRUSS AT BREEZEWAY WALL  
 3/4\"/>



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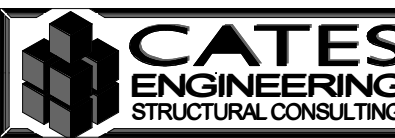
**POOLE & POOLE ARCHITECTURE**  
4240 Park Place Court  
Glen Allen, Virginia 23060  
Telephone 804.225.0215  
www.zpa.net

Project: 681-224-25  
CADD File:  
Drawn By: RCR/MRP  
Checked By: MRP

Permit Release:  
February 19, 2026  
Construction Release Set:  
-

Revisions
No.    Date    Description

ASI / RFI Revisions
No.    Date    Description



CATES ENGINEERING LLC HAS REVIEWED THIS SET OF DRAWINGS FOR THE PROJECT AND HAS NOTED ANY CHANGES TO THE PROJECT. THESE CHANGES ARE NOTED IN THE PROJECT RELEASE SET. THE PROJECT RELEASE SET IS THE FINAL SET OF DRAWINGS. SEE DIMENSIONS.

**Daleville Town Center Apartments III**  
an Apartment Community by  
Daleville Town Center Apartments III, LLC  
in Daleville, Virginia

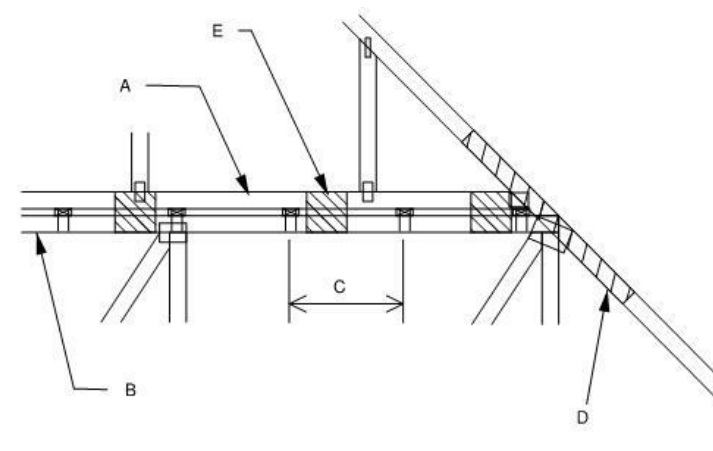
Drawing Title:  
**PITCHED ROOF FRAMING  
DETAILS**

**S5.5**

RELEASED FOR PERMIT

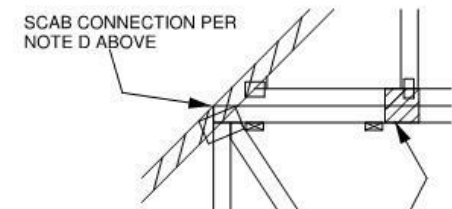
#### STANDARD CONNECTION DETAIL

- A. PIGGYBACK TRUSS SHALL BE CONNECTED TO EACH PURLIN  
B. BASE TRUSS  
C. PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM OF 24" O.C. UNLESS NOTED OTHERWISE. CONNECT TO BASE TRUSS WITH (2) 0.131X3" NAILS  
D. 2 x 4 x 4" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS. MIN. GRADE #2. ATTACHED TO ONE FACE, CENTERED ON INTERSECTION WITH (2) ROWS OF 0.131X3" NAILS AT 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1'-0" IN BOTH DIRECTIONS AND:  
a. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR  
b. WIND SPEED OF 110 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12'-0"  
E. FOR WIND SPEED IN THE RANGE 125 MPH - 160 MPH ADD 2" x 2" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH (3) 0.113X2" NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL 12 NAILS)



#### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

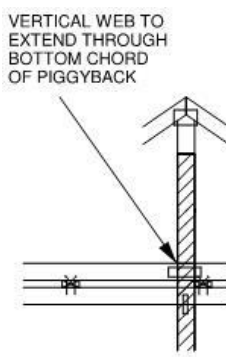
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING.



7' x 7' x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH (3) 0.113 X 2" NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

#### FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

1. VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE AND MUST LINEUP AS SHOWN IN DETAIL
2. ATTACH 2 x 4 x 4" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 0.131X3" NAILS SPACED AT 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS - MINIMUM 2x4)
3. THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS.
4. FOR PIGGYBACK TRUSSES CARRYING ORDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5. CONCENTRATED LOAD MUST BE APPLIED TO BOTH PIGGYBACK AND BASE TRUSS DESIGN.



ROUGH OPENING	END NAILS FROM KING STUDS TO ROOF HEADER	HARDWARE FROM TOP PLATES TO KING STUDS	FIGURE
≤4'-4"	(4) 0.131x3"	N/A	
≤8'-4"	(6) 0.131x3"	N/A	
≤9'-4"	(8) 0.131x3"	SIMPSON H8 OR (2) SIMPSON SDWC15600 SCREWS	3 OR 4

NOTES: SEE S0 2 SCHEDULE FOR TOP PLATE TO HEADER NAILING REQUIREMENTS

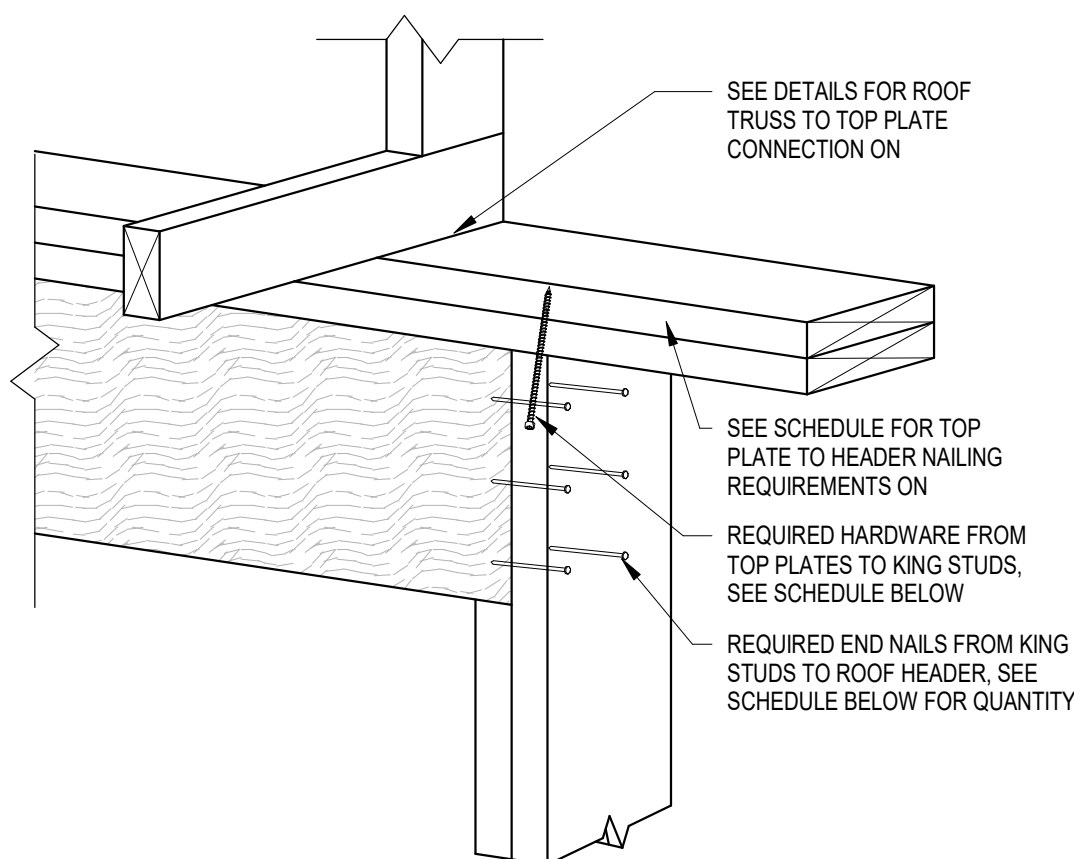


FIGURE 2

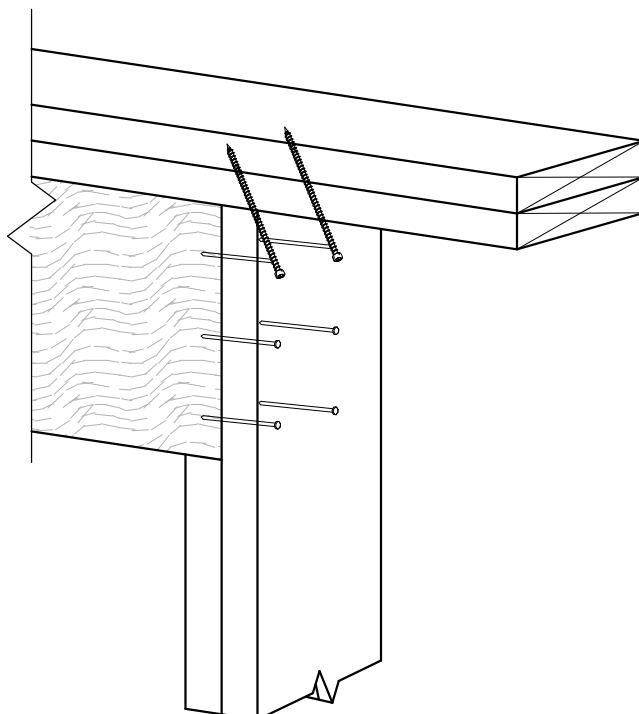


FIGURE 4

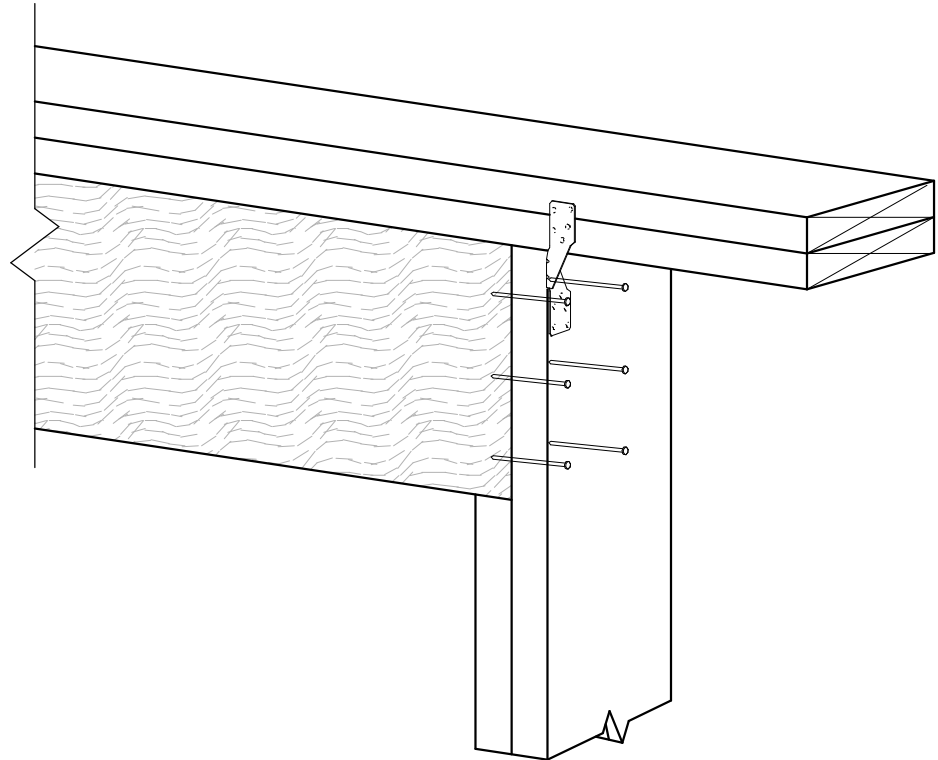


FIGURE 1

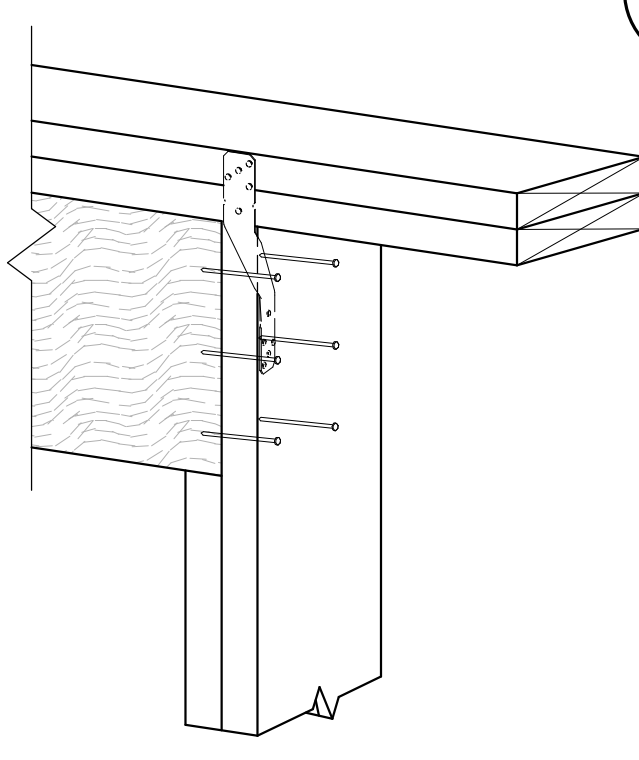
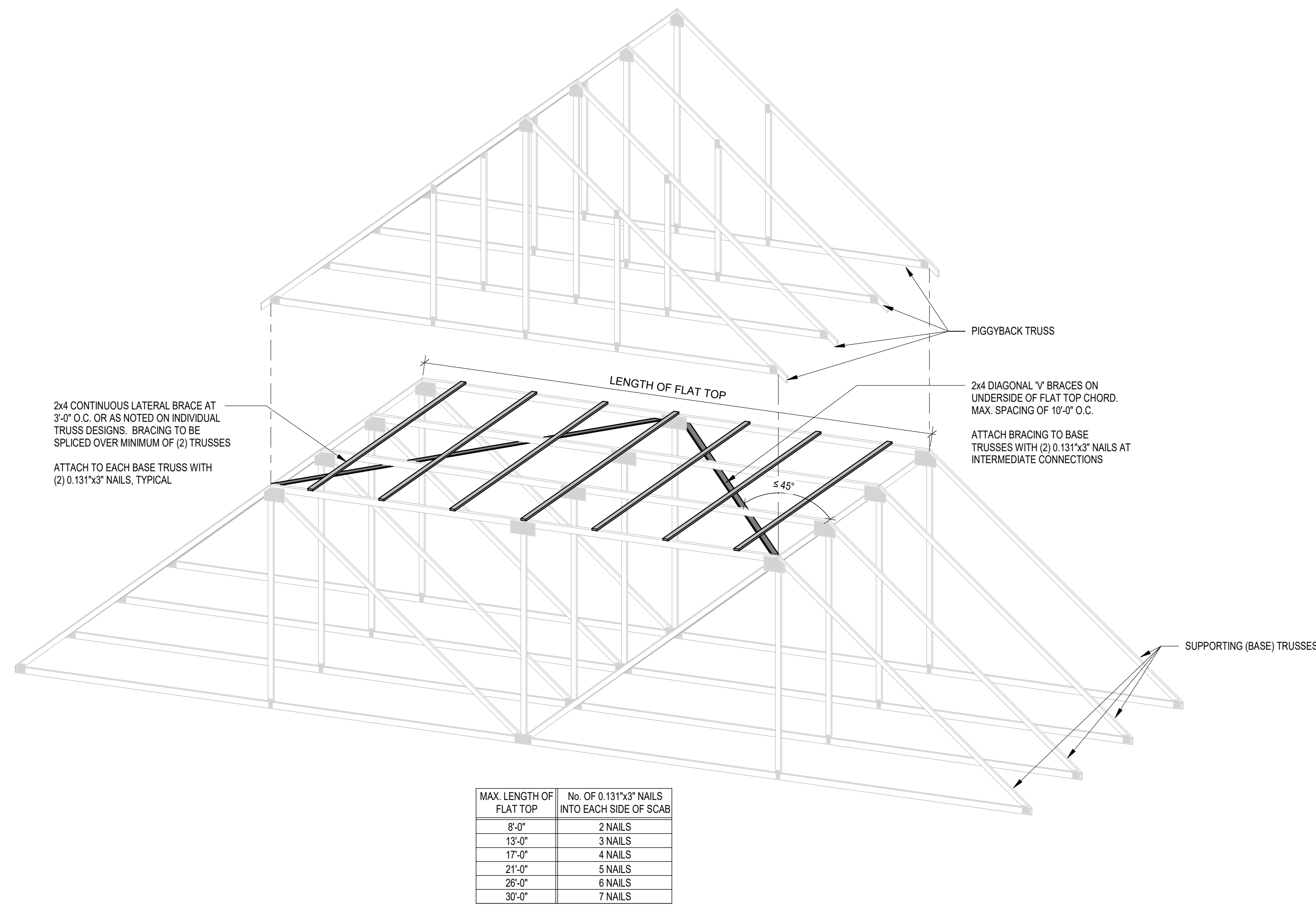


FIGURE 3

### 3 S5.5 ROOF TRUSS UPLIFT CONDITIONS

SCALE: N.T.S.

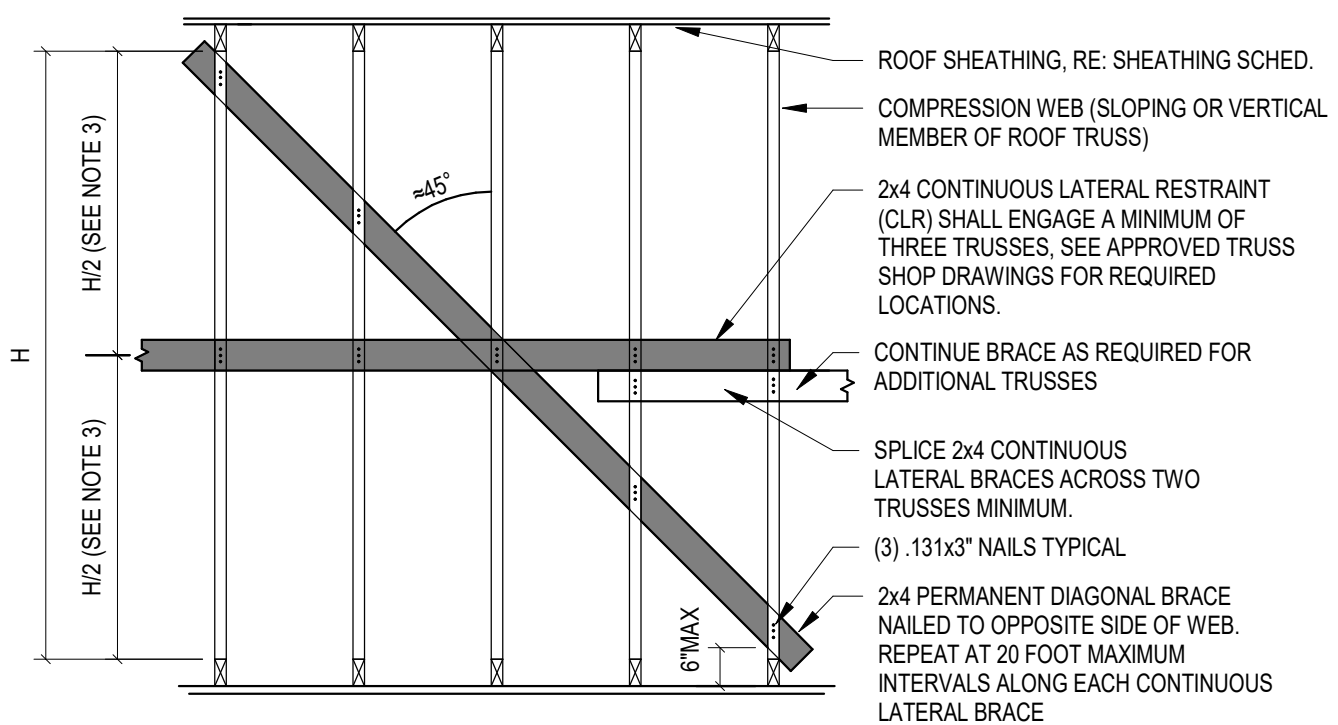


MAX. LENGTH OF FLAT TOP	No. OF 0.131X3" NAILS INTO EACH SIDE OF SCAB
8'-0"	2 NAILS
13'-0"	3 NAILS
17'-0"	4 NAILS
21'-0"	5 NAILS
26'-0"	6 NAILS
30'-0"	7 NAILS

### 1 S5.5 PIGGYBACK TRUSS CONNECTION

SCALE: N.T.S.

OPTION 1



- NOTES:  
1. DETAIL SHOULD ONLY BE USED WITH A MAXIMUM 30 PSF ROOF LIVE LOAD AND 90 MPH WIND LOAD.  
2. DIAGONAL BRACES SHALL TERMINATE WITHIN 6" OF A SHEATHED TRUSS CHORD.  
3. INTERVAL BETWEEN CONTINUOUS LATERAL BRACES SHALL BE W3 WHEN 2 BRACES ARE REQUIRED.

### 2 S5.5 TYPICAL PERMANENT DIAGONAL BRACING

SCALE: N.T.S.

IN THE PLANE OF ROOF TRUSS WEB MEMBERS